# Comparative Study of Design of Water Tank With Old And New Provision

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Abstract- Water is considered as the source of every creation and is thus a very crucial element for humans to live a healthy life. High demand of Clean and safe drinking water is rising day by day as one cannot live without water so design of water tank is safe. As per the provisions of the code (IS 3370-1965), the designing of water tanks was permitted by working stress method only and on the philosophy of no cracking. This code has been revised in 2009. As per IS 3370:2009, use of limit state method has been permitted and provision for checking the crack width is also included in this code. Hence this study was undertaken to compare the provisions of IS 3370: 1965 and IS 3370: 2009.

*Keywords*- IS 3370:1965, IS 3370:2009, Working Stress Method, Limit State Method, Crack Width Theory

#### I. INTRODUCTION

Water is the life line facility that must remain functional following disaster. Most municipalities in India have water supply system which depends on elevated tanks for storage. Elevated water tank is a large elevated water storage container constructed for the purpose of holding a water supply at a height sufficient to pressurize a water distribution system. Water storage tanks are designed as per the provisions of IS 3370. As per the provisions of the code (IS 3370-1965), the designing of water tanks was permitted by working stress method only and on the philosophy of no cracking. This code has been revised in 2009. As per IS 3370:2009, use of limit state method has been permitted and provision for checking the crack width is also included in this code. Hence this study was undertaken to compare the provisions of IS 3370: 1965 and IS 3370: 2009. Prasad and Kamdi (2012) had given effect of revised IS 3370 on water tank and concluded that thickness of wall and width of base slab is different for both codes because the value of permissible stress in steel is different and also concluded design of water tank by LSM is most economical as the quantity of material required is less as compared to WSM. Bhandari and Karan Deep Singh (2014) gives the comparison of IS 3370:1965 and IS 3370:2009 for WSM and LSM and other aspects. Design of three different types of water tank with reference to the IS 3370:1965 and IS 3370:2009 with different capacities. After concluded the design of water tank is most economical in LSM as compared to WSM and the quantity of material required is less in LSM. Lodhi, Sharma, Garg (2014) Design of intze water tank as per IS 3370:1965 without considering earthquake forces and after redesign the intze water tank with same parameter as per IS 3370:2009 with considering earthquake forces and concluded that design of intze water tank as per old IS code was unsafe in hoop tension. With considering earthquake forces in design of intze water tank the thickness of cylindrical wall, conical dome and bottom dome is increased. As per new IS code required reinforcement is also increases. Jindal and Singhal (2012) compared the IS 3370:1965 and IS 3370:2009 code of practice for concrete structures for the storage of liquids. It gives the comparison of WSM and LSM.

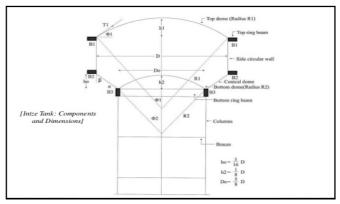


Fig.1.1 Components of Intze WaterTank

#### **II. OBJECTIVE**

- 1. To study the analysis and design of water tank.
- 2. To check about design philosophy for safe design of water tank.
- 3. To check economical design of water tank.
- 4. This report is to provide guidance in the design and construction for various types of water tanks.

#### **III. PERFORMANCE ANALYSIS**

### 3.1 Design Steps

#### 3.1.1 Underground rectangular tank

Design constants,

$$k = \frac{mc}{mc + \sigma_{st}}$$

Design of long wall,  $P_{\alpha} = K_{\alpha} \gamma' H + \gamma_{w} H$ 

Design of long wall,  $P_{\alpha} = K_{\alpha} \gamma'(H-h) + \gamma_{w}(H-h)$ 

Design of bottom slab,

$$P_{u=} w H_1$$

Where, m = modular ratio

**3.1.2 Tanks resting on ground** Design constants,

$$k = \frac{mc}{mc + \sigma_{st}}$$

Water pressure,  

$$P = w (H - h)$$

Cantilever moment,

$$= wH \times \frac{h^2}{6}$$

Reinforcement at corners of l ong walls,

 $X = d - \frac{T}{2}$ Where, m = modular ratio C = compressive force.  $\sigma_{st} = \text{permissible stress in s teel in tension.}$ H = Height.

W = Total load

d = Effective depth

T = Torsional moment

#### 3.1.3. Overhead tank

Dimensions of tank,

 $= \frac{\pi \times D^2}{\text{Max. Holop tension at base of wall}}$ 

$$F_1 = \frac{whD}{2}$$

Design of bottom spherical do me

$$R = \frac{\frac{D^2}{2} + r^2}{2r}$$

Total design load on the ring girder

$$W = \pi D w$$

Where, D = Diameter at base R = Radius of the dome r = central rise w = density of water h = depth of water W = Design load

#### **Assumptions of Design Water Tank**

Capacity of tank = 1 million litres = 1000 m3 Height of supporting tower = 16 m Number of columns = 8 Depth of foundations = 1 m below ground level.

## **IV.COMPARISION**

### **Comparative Result Of Ground Type Of Water Tank**

Structur al	WSM				LSM			
Element	IS 3370 – 1965		IS 3370 – 2009		Crack Theory		Deemed to Satisfy	
Top Dome								
Area of Required	Steel	300r	mm <sup>2</sup>	300mr	m <sup>2</sup>	120m m <sup>2</sup>	l	-
% of change		-		Nil		-60%		-
Thickness Required		100mm		100mm		100m m		100mm
% of Change		-		Nil		Nil		Nil
Roof Ring Beam								
Area of C/S	325000m m <sup>2</sup>		325000m m <sup>2</sup>		34500mm 2		34500mm 2	
% of Change	-		Nil		-89.39%		-89.39%	
Area of Steel Require	3484mm <sup>2</sup>		3484mm <sup>2</sup>		1801.62m m <sup>2</sup>		4019mm <sup>2</sup>	

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d					Require d					
% of Change	-	Nil	-48.29%	15.35%	% of Change	-	Nil	Nil	Nil	
Cylindrica	al Tank Wal	l			Top Ring	Beam	1	1	I	
Thickne ss	260mm	260mm	120mm	120mm	Area of C/S	105000m m <sup>2</sup>	105000m m <sup>2</sup>	34500mm <sup>2</sup>	34500mm	
% of Change	-	Nil	-53.85%	-53.85%	% of		- 11.43%	- 67.14%	- 67.14%	
Steel At Base	$\frac{2804.4\text{m}}{\text{m}^2}$	2804.4m m <sup>2</sup>	1351.72m m <sup>2</sup>	3015.39m m <sup>2</sup>	Change	-	- 11.4570	- 07.1470	- 07.1470	
% of Change	-	Nil	-51.8%	17.52%	Area of Steel Require	1001.32m m <sup>2</sup>	1155.31m m <sup>2</sup>	517.90mm 2	1155.31m m <sup>2</sup>	
Base Slab					d % of Change	-	- 15.38%	- 48.28%	- 15.38%	
Thickne ss	260mm	260mm	260mm	260mm	Cylindrical Tank Wall					
% of Change	-	Nil	-60%	-60%	Base					
Area of Steel Require	780mm <sup>2</sup>	780mm <sup>2</sup>	360mm <sup>2</sup>	-	Level Thickne ss	360mm	360mm	180mm	180mm	
d % of	_	Nil	-53.85%		% of Change	-	Nil	- 50%	- 50%	
Change					Steel At Base	3966.67m m <sup>2</sup>	$\begin{array}{c} 4576.92m\\m^2\end{array}$	2051.72m m <sup>2</sup>	$\begin{array}{c} 4576.92m\\m^2\end{array}$	
Comparat Structur	tive Result O WSM	of Intze Type	Of Water Ta	ank	% of Change	-	15.38%	- 48.28%	15.38%	
al Element	IS 3370 – 1965	IS 3370 – 2009	Crack Theory	Deemed to Satisfy	Top Level Thickne ss	200mm	200mm	100mm	100mm	
Top Domo	9				% of Change	-	Nil	- 5 0	-50%	
Area of Steel Require	300mm <sup>2</sup>	175mm <sup>2</sup>	120mm <sup>2</sup>	-	Steel At Top	700mm <sup>2</sup>	807.69m m <sup>2</sup>	% 586.20mm 2	807.69m m <sup>2</sup>	
d % of	<u> </u>	- 41.66	- 60%	-	% of Change	-	15.38%	-16.26%	15.38%	
change Thickne ss	100mm	% 100mm	100mm	100mm	Bottom Ring Beam					

Area of	720000m	720000m	720000m	720000m
C/S	m <sup>2</sup>	$m^2$	$m^2$	m <sup>2</sup>
% of Change	-	Nil	Nil	Nil
Area of Steel Require d	6885.67m m <sup>2</sup>	7280.46m m <sup>2</sup>	3263.66m m <sup>2</sup>	7280.46m m <sup>2</sup>
% of Change	-	14.56%	-22.88%	-15.38%

**Conical Dome** 

Thickne ss	600mm	600mm	600mm	600mm	
% of Change	-	Nil	Nil	Nil	
Area of Steel Require d	6309.73m m <sup>2</sup>	7280.46m m <sup>2</sup>	3263.66m m <sup>2</sup>	7280.46m m <sup>2</sup>	
% of Change	-	15.38%	-48.28%	15.38%	

# **Bottom Spherical Dome**

Thickne ss	300mm	300mm	300mm	300mm	
% of Change	-	Nil	Nil	Nil	
Area of Steel Require d	1050mm <sup>2</sup>	900mm <sup>2</sup>	150.6mm <sup>2</sup>	265.38m m <sup>2</sup>	
% of Change	-	-14.29%	43.43%	-74.75%	

# **V. CONCLUSION**

Design of water tank as per IS 3370: 2009 by limit state method is most economical as compared to IS 3370:1965 by working stress method. Area of steel for reinforcement is decreases in LSM as per IS code. The thickness of wall is decreases in limit state method. The size of member of ring beam is also decreases in limit state method. The quantity of material required is less in limit state method as compared to

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working stress method. Crack width calculations done in limit state method.

The thickness of wall and depth of base slab is comes to different for IS 3370:(1965) and IS 3370:(2009) because of the value of permissible stress in Steel (in direct tension ,bending and shear) IS 3370:(1965) value of  $\sigma$ st is 150 N/mm2 and in IS 3370:(2009)  $\sigma$ st is 130 N/mm2. Design of water tank by Limit State Method is most economical as the quantity of material required is less as compared to working stress method Water tank is the most important container to store water therefore, Crack width calculation of water tank is also necessary.

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