Multistory Building Along And Across Wind Analysis

Dewla Mohmadramiz¹, Patel Monika²

¹Dept of Civil Engineering(structure) ²Assistant professor, Dept of Civil Engineering(structure) ^{1, 2}Sakanchand Patel engineering college, Visnagar, Gujarat

Abstract- Wind is a perceptible natural motion of air relative to earth surface especially in the form of air current blowing in a particular direction .The major harmful aspect which concern to civil engineering structures is that, it will load any and every object that comes in its way. Wind blows with less speed in rough terrain and higher speed in smooth terrain present study is based an to determine .Wind load is really the result of wind pressures acting on the building surfaces during a wind event.

This wind pressure is primarily a function of the wind speed because the pressure or load increases with the square of the wind velocity. Structural walls, or shear walls, are elements used to resist lateral loads, such as those generated by wind and earthquakes .The effect of gust factor method multistory building along wind and across wind analysis of IS 875 part 3 (2015) on difference H/B ratio and different terrain category for along and across wind analysis. There are several model analysis using ETAB-2016.

Keywords- wind, terrain category, along wind , across wind , ETAB 2016

I. INTRODUCTION

T Movement of air with respect to the earth surface is known as wind. Earths" rotation and terrestrial radiation differences are the major causes of wind. The effects of the radiation are mainly accountable for either upward or downward convection. Generally at high wind speeds, the wind blows to the ground horizontally.

Vertical components of atmospheric motion are comparatively small. Thus the term wind almost exclusively means the horizontal wind. The capability of a structure to withstand enormous pressure of the wind depends on geography, nearness of other hindrances to the flow of air and also depends on the characteristics of the structure. The combined action of internal and external pressure acting on the structure as whole determines the effect of wind on it. In all cases, the computed wind load acts normally to the surface to which they apply. Combined and separate effects of wind loads and imposed loads on vertical Mean plus a fluctuating component constitutes Wind velocity. Gust will be created if the momentary deviation of the fluctuating component occurs from the mean value. Both of these components of wind velocity depends upon the approach terrain and varies with the height. The irregular shapes and square RC 3-

 D bare frame structures are studied for dynamic wind load cases. Wind analysis has been conducted as per IS: 875(part 3)-2015. The FEM software package ETABS 16 has been used for the modeling and analysis of the RC bare frames. Storey drift, Storey displacement and their variations are analyzed for dynamic wind load cases.

II. DESIGN PROCEDURE

Design Wind Speed

Speed of the wind in the atmospheric boundary layer increases with increase in height from ground level to top level at a height called as the gradient height. The variation with height depends mainly on the terrain conditions. However, the speed of the wind at any height never remains constant. It has also been found easier to determine its instantaneous magnitude to an average value and a fluctuating component near this average value. Peak gust velocity remains constant over a short period of time, of about 3 seconds for basic wind speed and corresponds to mean heights in an open terrain above ground level. As mentioned in the code, our country is divided into six different regions. As far as the basic wind speed is concerned, the basic wind speeds in six regions are 33, 39, 44, 47, 50 and 55 m/s respectively. The basic wind speed shall be modified to include risk level, terrain roughness, height of the structure and local topography to get the design wind velocity,

Vz given as:

Vz= Vb. K1. K2. K3 (of IS: 875 (Part 3) - 2015)

Where,

VZ = Design wind speed at any height z in m/s

Vb = Basic wind speed for different zones K1 = Probability factor (risk coefficient) K2 = Terrain roughness and height factor K3 = Topography factor

1) Risk coefficient (K1):

The suggested life period to be assumed and the corresponding K1 factor for different class of structures as per IS: 875 (Part 3)

2) Terrain and height factor (K2):

Terrain categories shall be selected with due regard given to the effect of obstruction, which constitute the ground surface. The terrain category used in the design of structure varies depending on the direction of wind under consideration. Terrain in which a specific structure stands shall be considered as being one of the following terrain categories.

a) Category 1

Exposed open terrain with few or no obstructions in which the mean height of any object that surrounds the structure is less than 1.5 m. Open sea-coasts and flat treeless plains are included in this category.

b) Category 2

Open terrain having well scattered obstructions with heights usually between 1.5 to 10m. This is the criterion for measuring regional basic wind speeds and includes open parklands, airfields and undeveloped sparsely built outskirts of suburbs and towns. Open land adjacent to sea coast also comes under Category 2, because of the roughness of large sea waves at high wind speeds.

c) Category 3

Terrain with many closely spaced obstructions having the size of building structures this category includes well wooded areas and shrubs, towns and industrial areas full or partially developed.

d) Category 4

Terrain with plenty of large high closely spaced obstructions . This category includes large city centers, generally with obstructions above 25m and well developed industrial complexes.

3) Topography Factor (K3):

The basic wind speed Vb, considers general level of site above the sea level. This does not allow for local topographic features such as valleys, hills, cliffs, ridges or escarpments, which can significantly affect wind speed in their vicinity. The effect of topography is to accelerate wind near the summits of hills or crests of cliffs, escarpments or ridges and decelerate the wind in valleys or near the foot of cliffs, steep escarpments or ridges.

The effect of topography is of significant importance at a site when the required slope is greater than about 3° , and below that, the value of K3 may be taken as equal to 1.0. The value of K3 is confined in the range of 1.0 to 1.36 for slopes greater than 3° .

Design Wind Pressure

The design wind pressure at any height above mean level can be obtained by the following relationship between wind pressure and wind velocity:

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PZ=0.6 Vz<sup>2</sup>
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Where,

PZ = Design wind pressure in N/m2 at height z m VZ = design wind velocity in m/s at height z m

Wind Load on Individual Members: (IS: 875 (Part 3)

$$\mathbf{F} = (\mathbf{Cpe} - \mathbf{Cpi}) \mathbf{APz}$$

Where, Cpe = external pressure coefficient, Cpi = internal pressure- coefficient, A = surface area of structural or cladding unit and Pz = design wind pressure.

Parameters considered for the study

Number of Storey 30 Bottom Storey height 3m Storey height 3m Type of building use residential buildings Foundation type Isolated footing Soil type Medium Wind zone III Shape of buildings rectangular shape and square shape Material Properties Grade of concrete M30 Young's modulus of concrete, Ec

25.0*10⁶kN/m²

Grade of steel Fe 415

Density of reinforced Concrete

25 kN/m³

Poisson's Ratio of reinforced concrete

0.25

Member Properties

Thickness of slab 0.125m Beam size 0.45*0.75m Column size 0.85*0.85m Dead load (DL) intensities Floor finish on floors 1.5 kN/m2 Floor finish on roof 2 kN/m2 Live load (LL) intensities Live load on floors 3 kN/m2 Live load on roof 2 kN/m2

Linear Analysis

Bottom Storey height= 3m, Each Storey height= 3m

The maximum dimension of the building is above 50m, hence it is classified in to "Class C", and Terrain Category 1-4 has been considered for the bare frame models, k1=1 Slope below 30, k3=1, Where k2 value (IS: 875(part 3)-2015).

GUST FACTOR

A gust factor is defined as the ratio between a peak gust and mean speed over a period of time. It can be used to examine the structure of the wind along with other statistics. The magnitude of fluctuating component of the wind speed, called gust, depends on the averaging time. Gust factors are heavily dependent on upstream terrain conditions (roughness), and are also affected by transitional flow regimes (specifically, changes in terrain and the distance from the upstream terrain change to the measuring device), Anemometer height, stability of the boundary layer, and potentially, the presence of deep convection.

Wind load calculation as per IS: 875 (part - 3) - 1987 with gust factor method for 20 floors in zone-1 (33 m/s)

Time Period Calculation:

h = 60m (height of structure)dx = 40m (dx = plan dimension in X- direction) dy = 40m (dy = plan dimension in Y-direction) Tx = 0.09h / d (From page – 48) Tx = 0.853 sec Ty = 0.853 sec

* Along wind

1. $F_z=C_fA_e P_d G$ $F_z=$ design peak along wind load on the building! structure at any height z

2. P_d =design hourly mean wind pressure corresponding to $P_z=0.6V_z^2$ (N/m²)

 C_f = the drag force coefficient of the building! structure corresponding to the area Az

3.V z,H = $K_2 v_b$

K₂= hourly mean wind speed factor for terrain category

 $K_2=0.1423[\ln (z / z_{0i})](z_{0i})^{0.0706}$

4. r = roughness factor which is twice the longitudinal turbulence intensity, $I_{h,i}$

5. B_s = background factor indicating the measure of slowly varying component of fluctuating wind load caused by the lower frequency wind speed variations

$$\frac{1}{1 + \frac{\sqrt{0.26(h-s)^2 * 0.46bs}}{Lh}}$$

6. $L_{h=}$ measure of effective turbulence length scale at the height, *h*, in m

 $85*(h/10)^{0.25}$ for terrain category 1 to 3 70*(h/10)^{0.25} for terrain category 4 7. Φ = factor to account for the second order turbulence intensity

$$(g_h I_{h I} \sqrt{b_s})/2$$

8. H_s=height factor for resonance response

$$H_{s}=1+(s/h)^{2}$$

9. S=size reduction factor given by:

$$\frac{1}{[1+\frac{3.5\,fah}{vh}][1+\frac{4\,fbh}{vh}]}$$

10. E =spectrum of turbulence in the approaching wind stream

Π N/(1+70.8N²)^{0.833}

11. β = damping coefficient of to be building/structure Bolted steel/RCC structures β : 0.020

12. N= effective reduced frequency

 F_aL_h/V_{hd}

13. G_R=peak factor for resonant response

$$\sqrt{[2\ell n 3600 fa]}$$

14.G= Gust Factor and is given by

 $G=1+r\sqrt{[g_v^2 B_s(1+g)^2+H_s g_R^2 SE/\beta]}$

* Across wind

1.g_{h =}a peak factor

$$\sqrt{[2\ell n 3600 fa]}$$

2. $\mathbf{P}_{\mathbf{d}}$ =design hourly mean wind pressure corresponding to

$$P_z = 0.6 V_z^2 (N/m^2)$$

3. V z,H = $K_2 v_b$

 K_2 = hourly mean wind speed factor for terrain category

$$K_2 = 0.1423[\ln (z / z_{0i})] (z_{0i})^{0.070}$$

4. b = the breadth of the structure normal to the wind, in m; h = the height of the structure, in m;

5. f_a = first mode natural frequency of the building! structure in across wind direction, in Hz.

6. k = a mode shape power exponent for representation of the fundamental mode shape as represented by:

 $(z/h)^k$

7. C_{fs} = across wind force spectrum coefficient generalized for a linear mode

Is 875 -2015 page no. (49,50)

8.across wind design peak base bending moment= M_c

$M_c = 0.5g_h p_h bh^2 (1.06-0.06k) (\Pi C_f)^{0.5}$

9. The across wind load distribution on the building structure can be obtained from $M_{\rm c}$ using linear distribution of loads as given below

$$F_{z,c} = (3M_c/h^2)(z/h)$$

Modeling in ETABS 16

Rectangular model











Wind load calculation with gust factor for 30 floors rectangular shape in as per IS: 875 (part-3) – 2015 terrain category 1 (along wind)



Wind load calculation with gust factor for 30 floors rectangular shape in as per IS: 875 (part-3) – 2015 terrain category 1 (across wind)

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Dynamic analysis

Wind load calculation with gust factor for 30 floors rectangular shape in as per IS: 875 (part-3) – 2015 terrain category 2 (along wind)

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Wind load calculation with gust factor for 30 floors rectangular shape in as per IS: 875 (part-3) – 2015 terrain category 3(along wind)

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Wind load calculation with gust factor for 30 floors rectangular shape in as per IS: 875 (part-3) – 2015 terrain category 2 (across wind)

Wind load calculation with gust factor for 30 floors rectangular shape in as per IS: 875 (part-3) – 2015 terrain category 3(across wind)

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175.7	17	67	15	0.511	3747	100				
185.F	19	풀니	125	0,7365	370	1.000	÷			
19b F	-19		125	0.7422	39. Y	1.00	ļ	į		•••
109 P	19	715	125	0.7475	31	499		-		• •
INF	11	AU	1.6	0,7225		11077	-	-	-	-
Inf	19	773	1.6	0.1313		182	ļ		••	-
1365	19	-	1.6	0.7EL	333.5	1.0	-	-		
145 F	19	65	145	0.7564	337.0	1.00	-		-	
1312	15	E 4	125	0.7705	33.15	1.0	-			· · ·
185 F	19	3	1.6	213	3999	1876	-			••
1787	19	17.2	1.6	0.37	317	176	-		-	•••
IBSP.	19	161	1.6	0.78	3	432	ļ		_	•
1912	15	1	1.6	0.7868	3921	120	-	i.		
308.F	10	182	145	0.305	3111	1.77		-	-	

Wind load calculation with gust factor for 30 floors rectangular shape in as per IS: 875 (part-3) – 2015 terrain category 4(along wind)

							Min t	•		a hat	
							-	- 200	lete i	I	Т
7he	144	100	724	T	Tτ	h	1	a:		ی بعد	lary the
La POD	41	42	21	0.3857	120	97	2012	75		785	IEN
a dPCD	41		21	्रतन		122	034	787	270	3 17	HEI
GRFL	41	85	21	0.212	7215	-	01D	27		38 9	725
12F	41	EL.	21	282	AR	193	0195	Т		48.5	735
25	22	82	LUTS	815	77872	155	0222	78	779	32.7	HØ5
367	15	75	LUTS	0.805	222	-	0230	20	ш	33 2	124
487	1.9	22		11110	7.	-	005	78	m	3374	122
18.F	-25	34		262	7.	-	2167	73	20	383	1913
68.7	2.0	33		5.64.2	78.0	-	22	755	25	334	211
78.2	28	312		122.9	775	-	81%	72	29	38.5	211
TR.F	15	31	LIG	0.63	781	-	222	28	22	-	2815
S&F	25	4	LIG	0.63	1	125	024	2	2	-	77.2
1.83	1.5	-10		1.55		101	104		23	-054	255
112.F	28	41	LIG	183		100	104	35	27	-82.1	736
Ch.F.	15	47	un	258	1	125	223	35	755	417	75.
DAF	25	35	un	20.0	-	1	10E	æ	234	-012	781
H&F	1.0	315	un	833	33	-	613	2	75	-012	711
URE	25	34	LIG.	0.718	363	-	212	73	72	-84	76
HAF	17	413	LUTS	127.0	25	-	022	73	73	453	775
TRE	25	422	ш	0.30	2012	-	0775	797		-	755
ELF.	15	6 1		0.36	27.	-	(III)	255	-		202
DE.F	38			0.142	37.5	-	01.16	739	2	-56	767
ZEF	25	23		0.43	37	-	0.0	755	-	-81	221
1mF	2.8	л		0,72		•	如何	792	-		7
12dF	15	27		0.33		-	0100	73	35	585	785
DEF	3.8	86		127.0	#15.B	-	010	76	28	323	755
2aF	25	15	us	0.764	33 77	-	020	77	26	527	734
ZAF	2.8	-	us	0.1108	-	-	20	Ш	-	521	721
38F	15	83	us	0.13	399	-	619	75	2	5874	78.7
THE	3.8	122	us	073	377	-	0192	729	20	385	222
38F	25	161		0.75	3	112	0.0	775	79	586	785
3tF	2.8	1	UB	0.75	382	-	012	ш	222	5761	211
24.7	10		LIG	0.70		175	012	77	755	384	7764

Wind load calculation with gust factor for 30 floors rectangular shape in as per IS: 875 (part-3) – 2015 terrain category 4(across wind)

Wind load calculation with gust factor for 30 floors square
shape in as per IS: 875 (part-3) – 2015 terrain category
1(along wind)



Wind load calculation with gust factor for 30 floors square shape in as per IS: 875 (part-3) – 2015 terrain category 1(across wind)

									I	1
The	100	100	37 HJ	E	Tt	h	Ba.	3 7	-	
1#900	14	42	21	0326		11757				-
IN POL	41	L	21	\$4142	1.		ļ		-0.0	ġ
GRR	41	25	บ	0316	<u>315</u>	1307		ļ	1	Ë
IZ.F.	41	5	บ	0.362	172	137	i	Ì		÷
Indf	19	2 7	1.5	0.23	7472	1.74	Ì	į		į
End F	10	25	1.5	0.600	222	112	ļ	İ		
4182	19	5 5	1.5	2612	795	186	ļ	į	i	ļ
1thE	19	11	1.5	0.628	79	186	ļ		- 70	ļ
fthF	19	33	1.5	0.64 1	78.04	UTD	ļ		ġ	ļ
THE.	10	312	1.5	1508.0	175	1467	ġ		93	Ē
EthF	19	31	1.5	0.6634	781	1112	ļ			
SthE	19	4	1.5	263	79	1.78	ļ	i	ļ	ţ
101.F	19	-10	1.5	0.65 1	-	ļ	į	İ	÷.	Ė
186F	10	-51	1.5	0.690	337	1366		i		ţ
128F	19	417	1.5	151	3.7	111	i		20 L	ļ
135.7	19	35	1.5	0.1055	39	1.777	i			
145 F	19	35	1.5	2,712	337	1.26	ļ			į
135 F	10	31	1.5	0.71m	39	1	ļ			
186F	19	-	1.5	0,725	376	186	ļ			ţ
THE	19	#1	1.5	0.1211	316	4.28	ļ	İ	l	i
1257	19	4 1	1.5	0,736	37.0	1.00	ļ	ļ		i
195F	10		1.5	2,742	3747	1.00	ļ	ļ		÷
10b F	19	33	15	27472	3	199	ļ	ļ		
1127	19	71	1.5	6.752	포케	4677	ļ	ļ	į.	ļ
INT.	19	27	1.5	0.7378	HU	100	-	-		-
136F	10	-	1.5	2751	H 1	1.24	-	-		
145 F	19	65	1.5	0,7668	377	1.001	-	-	-	
13b P	19	E.	1.5	0.110	3176	1.01	-	-		•
1957	19	1.3	15	0.775	300	187	-0000	-		÷
THE	15	122	15	272	3 77	175	ļ	-		
18HF	TP	151	15	0.73	30	432	-	ė		
199 F	19	1	15	817.0	342	121	-	ļ		
30% P	20	HER	15	0.790	3.72	1.77	-			

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1#POD	41	-		0.25		873	784	-	-	1.
Int POE	41			644		1052			-	
GREL	4	125		220	<u>315</u>	8	ļ	Į	•	Ĭ
12 F	4	ны		0.57	17	8		į		ġ
INT	25	11.7	1.6	0.533	7452	1974		10001	1	100
3ndF	19	775	1.6	0 H C	200	-			-	-
4thF	29	<u>55</u>	1.65	06135	THE	-			-	
diar.	25	214	145	062	27 9 2		-			12.000
SOF.	27	33	1.65	05415	78.0	-				
TIME	19	312	1.6	16231	177	-		-		
Star	15	Ξ	1.6	1634	781	1010				
StaF	19	4	1.6	0.613	39	111				
101.F	27	423	1.6	0.00	312	121				
BBF	15	-61	1.6	080	33	1000			-	
128 F	19		1.6	9.675	3.7	100			22	
1387	15	34	1.6	075	39	177				
145F	19	385	145	110	33	125				
125.7	29	524	1.65	2112	3.9	-	-			-
193 F	29	-	1.6	1207.0	35	-				
1762	29	62	1.6	030	3H2		•			
13hF	19		1.6	076	37.	-				
1917	29		1.6	171	364	-	Ĩ		ì	
1017	22	719	1.6	07473	37	1.0		••••	-	
TINE	25	AL	1.6	072	포케	100	Ĩ	-	-	
Intr	19	717	1.6	100	3 3				-	11.7
1365	18	-	1.6	0.762	355		-			
148 F	19	65	1.6	135	37	-	-		-	-
135 F	27	E.	1.6	0705	395	-	-		-	
288F	19	-3	1.6	211.2	310	-	-	-	9.2	-
THEF	19	17.2	1.6	0.779	3 17	18	-			-
185 F	19	161	1.6	0.73	310	1013	-		-	
195.F	27	1	1.6	0119	342	1011			-	-
104.2	1.00	1000	1.05	1. 19.00			200			



Wind load calculation with gust factor for 30 floors square shape in as per IS: 875 (part-3) – 2015 terrain category 2(along wind)



Wind load calculation with gust factor for 30 floors square shape in as per IS: 875 (part-3) - 2015 terrain category 2(across wind)

Wind load calculation with gust factor for 30 floors	square
shape in as per IS: 875 (part-3) - 2015 terrain ca	ategory
3(along wind)	

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la Ric	42	42	21	0387	1200	1171	1220	Z	7.	44	3	6 K3
er POL	42		21	94747		12	077	22	72	15.1		H 103
GRE.	41	25	21	05252	79 5	127	0257	23	73	65	m	DR- 05
ist	41	•	21	2068	7972	13	02.03	23	73	827		CRY- 03
345	18	2	LÆ	8,55	7993	135	014	23	73	25	672	
367	2.8	25	LÆ	66003	782	411	014	2.4	7.	1 59	8 2	nem indl
49F	28	<u>55</u>	LÆ	06128	7.	-	0.4	2.6	75	Z	-	uran Said4
31F	25	24	LÆ	06233	7	-	0237	2.6	75		79	5- 4
852	15	33	LE	06418	78.0	1 10	2134	28	78	83	72	The Paper
THE.	2.0	32	1.5	6611	70.05	155	6731	2.0	7.8	056		
THE	28	31	1.5	0104	781	111	813	7 3	73	81		
915	28		LE	0.65	. ·	13	22.25	25	75	86.2	86	
ICE.F	2.0	-10	LE	06812	-	1.	012	25	75	82		
112.F	25	-	LE	66701	1.00	15	6111	23	73	164		
CAF	11	-	LAN	262	2.07	13	02.17	24	7	R67	11.7	
GRF	18	15	LE	07033	3455	171	8217	282	782	83	19	
H&F	2.0	35	LE	61.03	333	13	02.0	25	75		192	
UnF	2.5	24	LE	DTHES	363	-	01.0	28	78	121	192	
S&F	28	-	LAN	0121	3165	111	01.0	28	79			
TRE	28	# 2	LÆ	07311	3768	460	22.10	22	72	11	- 77	
TA.F	20	6 1	LÆ	07368		4.00	010	28	725	- 17		
Wit F	28		LÆ	0140	17. Y	1.	0207	22	72	7	•	
InF	28	U	LÆ	01413	37	18	0206	Z	7.	EE 7		
luF	28	л	LÆ	07533		488	10.04	Z	72	655		
1267	2.0	22	LÆ	27573	****	485	0205	Z	75		IL 5	
Did F	28	•	L	275		1171	010	28	72	I		
28F	11	5	L	0164	111 7	482	0201	28	73	E 7		
28F	15	E.	L	9776	-	110	212	28	78	1 3	88	
38F	2.0	B	L	875		1.0	01英	25	75	-7		
DAF	28	122	L	217	3977	18	016	218	718]
INF	2.0	1	L	010		432	01%	25	75	1 2]
31F	28	1	L	1783		131	010	25	75	5	173	1
38.5	28	HER	LA	01925		1.75	018	75	75		197	J

Wind load calculation with gust factor for 30 floors square shape in as per IS: 875 (part-3) – 2015 terrain category 3(across wind)

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100 2.1 0.000 3.700 6300 <th< td=""><td></td></th<>	
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28.5 1.3 3.1 1.45 0.501 21.9 62.0 1.000 1.000 59.2 2.3 4.0 1.45 55.5 3.940 65.0 1.000 0.000 10.15 2.4 1.45 55.5 3.940 65.0 1.000 0.000 0.000 10.15 2.4 1.45 55.05 3.940 65.0 0.000 0.000 0.000 0.000 12.5 2.5 4.0 1.45 55.05 3.940 65.00 0.000	-
Str 13 13 14	ч.
101 17 47 1.6 530 360 650 650 500	
1987 23 4.8 1.8 9.80 2.37 4.95 500 500 2.50 1287 23 4.7 1.8 148 2.7 4.8 50 500 2.50	
1285 33 407 1.65 688 3.78 4580 4680 4688 2248	1.
130F 13 35 125 139 349 4577 488 488 888 888	
AND 10 10 10 10 10 10 10 10 10 10 10 10 10	
1985 32 24 145 0188 348 mm mm	-
1885 2.0 483 1.05 0.121 3.98 4887 4888 5858 4848	
1917 10 452 1.05 0191 2142 45 10 100 100 100	
1317 17 41 1.5 9138 2.8 435 440 FBB 835	
100F 20 40 1.05 0.742 2.50 430 200 400 40.00	
100F 10 25 1.5 0101 2.00 4.00 200 40.00 40.00	·
1977 10 71 125 0111 319 4577 300 000 CLAR	
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2267 23 B5 1.05 012 3.55 (S.W. Mart 1.00) 4.00	
1487 13 ES 1.5 0784 3.78 (SE) 200 USB 5040	
2016 23 ES 1.5 0100 205 400 200 200 200	
1995 15 93 15 073 349 485 380 GBS 330	-
1382 13 TEZ 1.05 (17) 3278 4.05 200 (200 S.M.)	
1212 12 10 125 2122 340 478 2200 4000 40.00	-
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	-	-			m		-		See	
12200	41	42	21	0,3867	13015	1	į	ļ	ġ	ļ
12900	41	м	21	0.4141	-	428 B	2	į	200	
GREL	41	12.6	21	0.5362	쒭뗤	4.917	i	ļ	ч.	į
bE.	41	HÉ.	21	0.805	200	4307	i	į		ļ
2445	-29	117	15	45首	262	5	i	ġ	1	1
3rdF	14	22.6	1.6	0,6005	262	448		Ì		ļ
當時	-20	255	15	6,6158	2365	-	Ì	i	1	1
SHE	92	254	1.6	0.6195	2387	100				-
tehr .	11	23	1.6	6433	2826	-				
THE	.19	912	1.6	0.6531	2075	4454		•		•••
2445	-28	571	1.6	6.6634	28.0	4312	i	:		
SHEF	-20		1.6	642	2601	4.96				
10hF	19	69	1.6	0.0819	10002	Ê	i		1.00	i
1MhF	15	-61	1.6	0.605	30.07	4.536	:	•••	2012	
121.F	10	-	1.6	0.692	50711	ŝ	į	i	72	
1967	-25	94	1.6	0.7053		4.979			20.00	
141.5	1.2	545	1.6	0.7123	1110	1.201	i			l
tabr	29	274	1.6	0.7182	203	4.002				
184F	2.0	8 3	1.6	6,2251	31965	4467				Ì
13bF	14	822	1.6	0.511	250	-		1.1		
(85 F	29	6	1.6	0.7562	100	1.005	i	i		
1927	14		16	12931	100	100			-	
199.7	120	713	15	0.7475	121	460	ļ	i	-0.20	ļ
11sF	22	20	16	0.315	226	4.077			-	
THEF	11	773	1.6	0.1513	20021	160	İ	i	-0.00	
3562	29	24	16	27.0	1006	-	-	1.1	-	
SALF	-28	85	1.6	0.7664	20724				50.00	-
194F	20	164	1.6	6.7766	20015	4.016				-
16b.F	1.9	B 3	1.6	6375	2000	4.0%	1		59.2	-
234F	15	122	1.6	0.73	50278	476		1.1		-
185F	10	151	1.6	0.125	5465	432			-0.9	-
192 F	-25	11	1.6	8.1865	56215	4.211			-	
1012	20	LINK	15	6 265	1	1.20				



Wind load calculation with gust factor for 30 floors square shape in as per IS: 875 (part-3) – 2015 terrain category 3(across wind)



Wind load calculation with gust factor for 30 floors square shape in as per IS: 875 (part-3) – 2015 terrain category 3(across wind)

									I	Т
R	344	12.4	лн	D	¥.	k	ļ	×.		
(#200	42	42	บ	0.3967	046	4.0797	-	-	-	
- 20C	41	ш	21	0.41et	2.0	4391		-	-	
GRE	45	24	บ	6.5140	215	4.917		-		2.00
la F	41	K.	บ	0.5822	202	1107		-		
IndE	129	87	15	6.935	38	1.94		10011	2	1.0
345	14	24	15	0.8005	20	448		ļ	Ī	Ė
465	-20	25	15	2160	765	196	ľ	I	I	l
SHE	92	14	1.5	0,6195	78	4.685				
Sel-F	11	13	1.5	0.64 (8)	2504	4475				-
THE	.19	922	1.5	0.850	275	4.051				
St.EF	-28	91	1.5	0,6654	280	4912	i			•••••
SHEE	-20		1.5	663	28	1.262		-		
10hF	19	40	15	6.66 %	-				-	
18hF	15	41	1.5	2.6902	2.25	4.524		•••		
12hF	10	-	15	0.651		4.52			22	
199.5	28	94	15	0,703	3.00	4.575				
14kF	19	95	15	0712	110	1.00		-		
19hF	29	94	15	0118	10	1002				
10hF	42	-	15	0.7258	3 26	4440				
174F	19	42	15	6731	216	-				
185 F	29	- 61	15	6.736	2.0	1005				
1927	19	•	15	0.7422	2.0	-	ļ		-	
19h.P	-29	1)	15	0,743	•	440	-			<u> </u>
linf	92	71	14	0.752	210	44877	-			
The F	18	22		0.7373	9.53	150	-			
3262	29	1	14	0.85	2.24					
242.5	28	1		8,7664	2.74					
19hP	-29			0.1106	316					
IBSF	1.9	-		0.753			_			
THEF	19	22		0.019	1000					
Jahr	1.9			0.03		C 22				
192F	42	1		0,728					_	
10000	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			THE OWNER WATCHING						



Rectangular plan:







Square plan:







III. CONCLUSION

- As the height of the model increases, deflection on top storey also increases
- The determine wind analysis study high rise building are done MS EXCEL as per is 875 2015 for this purpose wind loading in term of along and across analysis done different condition the results of the analysis are show below in this graph.

IV. FUTURE SCOPE

- Building analysis is done here by taking different condition height of building but it is suggested to exclusive experimental test building will be carried out in wind tunnel test to check and compare the analytical and experimental results.
- Infill walls may be considered and the effect of which can be observed.
- Analysis can be carried out for different heights and remaining wind zones.

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