

# Prediction of Compressive Strength of High Performance Concrete and It's Durability

Venkata Narsireddy Sagili <sup>1</sup>, T.Sravana Sandhya <sup>2</sup>

<sup>1,2</sup>Dept of Civil Engineering

<sup>1</sup>ArbaminchInstitute of Technology, Ethiopia

<sup>2</sup>College of Engineering &Technology, samara university, Afar,Ethiopia

**Abstract-** *Neural networks have recently been widely used in some of the human activities in many areas of civil engineering applications. The present study is relating to the models in artificial neural networks (ANN) for predicting compressive strength of cubes and their durability of concrete containing metakaolin with fly ash and silica fume with fly ash have been developed at the age of 3,7,28,56 and 90 days respectively. For purpose of development of these models, training and testing used based on the available experimental results for required number of specimens produced with 7 different mixture proportions were used. The data used in the multilayer feed forward neural networks (FFNN) models are arranged in format of eight input parameters that covers the age of specimen, cement, metakaolin (MK), fly ash (FA), water, sand, aggregate and super plasticizer and in another set of specimen which contain silica fume (SF) instead of metakaolin (MK). As per these input parameters, FFNN are used to predict the compressive strength and its durability. The training and testing results in the neural network models have shown that neural networks have strong potential for predicting 3,7,28,56 and 90 days compressive strength and its durability which contains metakaolin, silica fume and fly ash at 5% level of significance.*

## I. INTRODUCTION

High Performance Concrete is a term used to describe concrete with special properties. HPC was first known to be concrete with high strength for structural purpose. However, advances in concrete technology have generated a new (Super plasticizer, retarders, fly ash, blast furnace slag, silica fume, fumed silica and metakaolin) combined according to a selected mix design, properly mixed, transported, placed, consolidated and cured to give excellent performance, such as high compressive strength, high density, low shrinkage, high modulus of elasticity, low permeability, and good resistance. Also, the concrete must have a durability factor greater than 80 after 300 cycles of freezing and thawing to meet their definition,

The American Concrete Institute (ACI) formed a special committee on HPC in 1992. This committee has taken

a broader view of HPC to include performance aspects other than compressive strength in its definition (this definition is similar to an earlier definition proposed by the National Institute of Standards and Technology):

Concrete meeting special performance and uniformity requirements which cannot always be achieved routinely using only conventional constituents and normal mixing, placing and curing practices. These requirements may involve enhancements of the following:

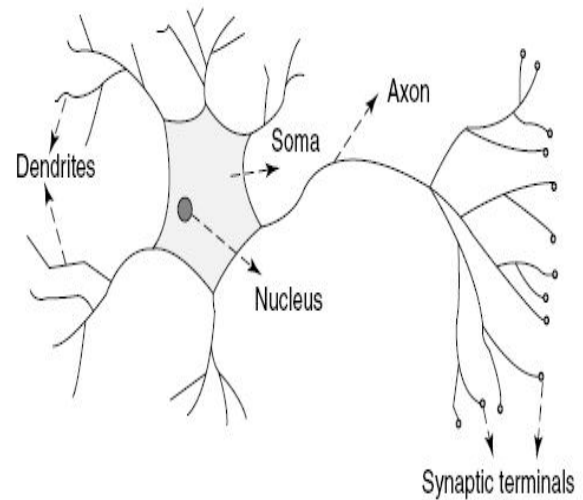
- ✓ Ease of placement and completion without segregation
- ✓ Long-term mechanical properties
- ✓ Early-age strength
- ✓ Toughness
- ✓ Volume stability
- ✓ Long life in severe environments

HPC can be designed to meet special performance requirements with regard to workability, strength and durability. To produce HPC, it is normal/general/necessary to use super plasticizing chemical admixtures in addition to the same ingredients, which are generally used for normal concrete. However, such HPC requires high paste volume, which often leads to excessive shrinkage and large evolution of heat of hydration besides, increase in cost. A partial substitution of cement by mineral admixture such as flyash, silica fume, Metakaolin, fumed silica and ground granulated blast furnace slag (GBS) is usually used. Use of such materials not only improves the properties of fresh concrete but also enhances the durability characteristics. When high strength is required, the use of micro silica becomes imperative, especially in the case of precast structural elements since micro silica achieves early strength even at 12 and 24 hrs. In addition, it improves impermeability, resistance to aggressive fluids, mitigation of alkali-aggregate reaction etc.

## II. METHODOLOGY

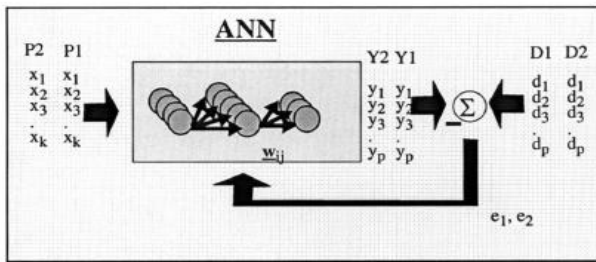
### 1. Neural Networks

An **artificial neural network (ANN)**, usually called **neural network (NN)**, is a mathematical model or computational model that is inspired by the structure and/or functional aspects of biological neural networks. A neural network consists of an interconnected group of artificial neurons, and it processes information using a connectionist approach to computation. In most cases an ANN is an adaptive system that changes its structure based on external or internal information that flows through the network during the learning phase. Modern neural networks are non-linear statistical data modeling tools. They are usually used to model complex relationships between inputs and outputs or to find patterns in data. An artificial neural network is an interconnected group of nodes, akin to the vast network of neurons in the human brain.



**Analysis:**

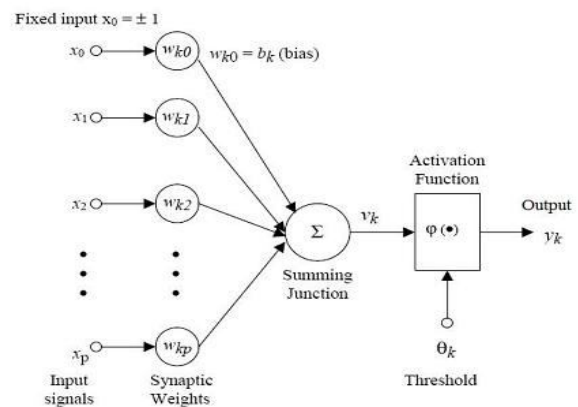
Once modeling an artificial functional model from the biological neuron, we must take into account three basic components. First off, the synapses of the biological neuron are modeled as weights. Let's remember that the synapse of the biological neuron is the one which interconnects the neural network and gives the strength of the connection.. Mathematically, this process is described in the figure



The style of neural computation.

**2. The Biological Model**

Artificial neural networks born after McCulloch and Pitts introduced a set of simplified neurons in 1943. These neurons were represented as models of biological networks into conceptual components for circuits that could perform computational tasks. The basic model of the artificial neuron is founded upon the functionality of the biological neuron. By definition, "Neurons are basic signaling units of the nervous system of a living being in which each neuron is a discrete cell whose several processes are from its cell body"



From this model the interval activity of the neuron can be shown to be:

$$v_k = \sum_{j=1}^p w_{kj} x_j$$

The output of the neuron,  $y_k$ , would therefore be the outcome of some activation function on the value of  $v_k$ .

**Activation functions**

As mentioned previously, the activation function acts as a squashing function, such that the output of a neuron in a neural network is between certain values (usually 0 and 1, or -1 and 1). In general, there are three types of activation functions, denoted by  $\Phi(\cdot)$ . First, there is the Threshold Function which takes on a value of 0 if the summed input is less than a certain threshold value ( $v$ ), and the value 1 if the summed input is greater than or equal to the threshold value.

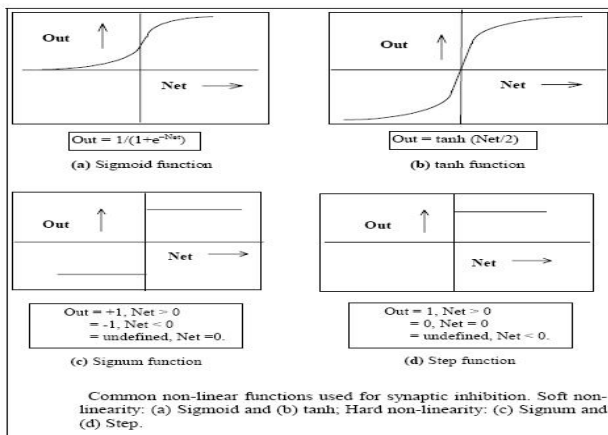
$$\varphi(v) = \begin{cases} 1 & \text{if } v \geq 0 \\ 0 & \text{if } v < 0 \end{cases}$$

Secondly, there is the Piecewise-Linear function. This function again can take on the values of 0 or 1, but can also take on values between that depending on the amplification factor in a certain region of linear operation.

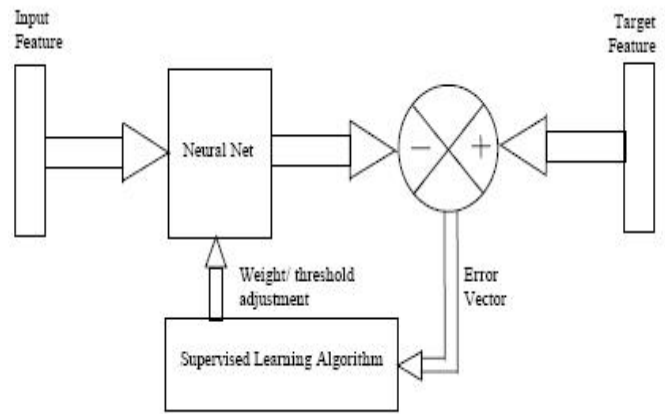
$$\varphi(v) = \begin{cases} 1 & v \geq \frac{1}{2} \\ v & -\frac{1}{2} > v > \frac{1}{2} \\ 0 & v \leq -\frac{1}{2} \end{cases}$$

Thirdly, there is the sigmoid function. This function can range between 0 and 1, but it is also sometimes useful to use the -1 to 1 range. An example of the sigmoid function is the hyperbolic tangent function

$$\varphi(v) = \tanh\left(\frac{v}{2}\right) = \frac{1 - \exp(-v)}{1 + \exp(-v)}$$



**Framework for distributed representation**



**Modifying patterns of connectivity of Neural Networks**

Both learning paradigms supervised learning and unsupervised learning result in an adjustment of the weights of the connections between units, according to some modification rule.

The basic idea is that if two units  $j$  and  $k$  are active simultaneously, their interconnection must be strengthened. If  $j$  receives input from  $k$ , the simplest version of Hebbian learning prescribes to modify the weight  $w_{jk}$  with

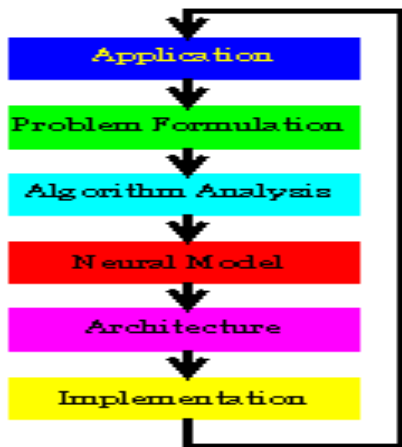
$$\Delta w_{jk} = \gamma y_j y_k,$$

where  $\gamma$  is a positive constant of proportionality representing the learning rate. Another common rule uses not the actual activation of unit  $k$  but the difference between the actual and desired activation for adjusting the weights:

$$\Delta w_{jk} = \gamma y_j (d_k - y_k),$$

in which  $d_k$  is the desired activation provided by a teacher. This is often called the Widrow-Hoff rule or the delta rule, and will be discussed in the next chapter. Many variants (often very exotic ones) have been published the last few years.

In order to have an integrated understanding on neural networks, we adopt the next perspective, called **top-down**, from application, algorithm to architecture:



The application domains of neural nets can be roughly divided into the following categories: association, clustering, classifications, pattern completion, regression and generalization, and optimization.

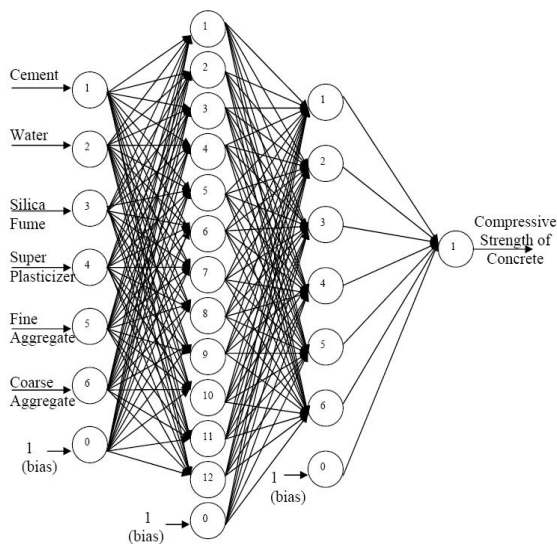


Fig 1. Architecture of the Developed ANN

**Experimental Programme:**

The testing process has been carried out for a total 140 data sets. Figure 6 exhibits the comparison between predicted compressive strength of concrete against the experimental evidence, which highlighted that there is a good agreement between the predicted values and that of experiment data. The results are shown that the artificial neural network was very successful in predicting of compressive strength of error with MSE of 6.02 percent. Also the ANN predicts the compressive strength of concrete in testing stage reasonably well the 6-12-6-1 neural models in general performs better than the others and it is able to give accurate prediction of compressive strength of concrete. In testing, result showed that, bad output (compressive strength with big error) usually occurred from the bad dataset in range (0-30). One

explanation for these results could be that there is an insufficient amount of training data around this range. The purpose of this experimental investigation is to study the behavior of High Performance Concrete (HPC). In this investigation HPC was manufactured by usual ingredients such as cement, fine aggregate, coarse aggregate, water and mineral admixtures such as Silica Fume (SF), metakaolin and Fly ash at various replacement levels and the Super Plasticizer used was CERAPLAST-300. The water binder ratio (w/b) adopted is 0.30 and 0.32. The concrete used in this investigation was proportioned to target a mean strength of 60 MPa. Specimens such as cubes, cylinders and prism beams were cast and tested for various mixes viz. seven mixes M1 to M7 are cast with 0%, 5%, 7.5% and 10% replacement of SF and another set of specimens with 0%, 5%, 7.5% and 10% replacement of SF along with 10% constant replacement of Fly ash and also metakaolin replacement same percentage of silica fume along with a constant replacement of fly ash to study the mechanical properties such as compressive strength, durability tests of concrete and ultimate load of beam and columns at different ages of concrete such as 3, 7, 28, 56 and 90 days.

Table 1. Compressive strength of concrete mixes with and without mineral admixtures with water binder ratio of 0.3 at various ages

Age in days	Compressive strength for 0.3 w/b (MPa)												
	NC	SF 5%	SF 7.5%	SF 10%	SF 5% FA 10%	SF 7.5% FA 10%	SF 10% FA 10%	MK 5%	MK 7.5%	MK 10%	MK 5% FA 10%	MK 7.5% FA 10%	MK 10% FA 10%
	3	35.67	32	34.33	32.67	33.67	31.33	29	38	41	39.67	29.67	41.33
7	42.33	40.67	44.67	40.33	42.33	41	39.33	39.33	49.33	43.33	37.67	46.67	43.33
28	54.67	55	61.33	56.33	58.67	57.33	55.33	55.67	59	57.67	56	64	55.67
56	61.33	61.67	69.67	65.67	68.67	68.33	60.33	64.67	66.33	62.67	65.33	67	62
90	66.67	67.67	76.33	71.67	74.33	73.67	67.33	72.33	77.33	71	73.33	80.67	72.33

Table 2. Compressive strength of concrete mixes with and without mineral admixtures with water binder ratio of 0.32 at various ages

Age in days	Compressive strength for 0.32w/b (MPa)												
	NC	SF 5%	SF 7.5%	SF 10%	SF 5% FA 10%	SF 7.5% FA 10%	SF 10% FA 10%	MK 5%	MK 7.5%	MK 10%	MK 5% FA 10%	MK 7.5% FA 10%	MK 10% FA 10%
	3	41	42	43	39	38	36.5	34.5	42.4	46	44.5	43	47.5
7	42.33	40.67	44.67	44.33	42.33	41	39.33	55.35	57.5	56.63	54.5	58.1	54.05
28	61	63	66	60	57	64	63.70	63.70	67	65.20	66	68.50	64.80
56	66	68	73.5	65	64	69	72.5	70.00	72.95	69.00	70.42	74.00	71.50
90	66.67	67.67	76.33	71.67	74.33	73.67	67.33	72.33	77.33	71	73.33	80.67	72.33

MATLAB is a high-performance language for technical computing. It integrates computation, regression, and iterations in an easy-to-use environment where problems and solutions are expressed in familiar mathematical notation. MATLAB was originally written to provide easy access to matrix software developed by the Linear PACKAGE LINPACK projects. MATLAB engines incorporate the LINEAR ALGEBRA PACKAGE (LAPACK) and Basic Linear Algebra Subprograms (BLAS) libraries, embedding the state of the art in software for matrix computation.

In this study, a multilayered feed forward neural network with a back propagation algorithm was adopted. The nonlinear sigmoid function was used in the hidden layer and the cell outputs at the output layer. As seen in Figs. 2 and 3 two different multilayer artificial neural network architectures namely ANN-I and ANN-II were built. In training and testing of the ANN-I and ANN-II models constituted with two different architectures AS, C, SF,FA,W, CA,S and SP were entered as input of first set and metakaolin used instead of silica fume as another set of inputs ; while  $f_c$  value was used as output. In the ANN-I and ANN-II, 140 data of experiment results were used for training whereas 65 of these data were employed for testing. In ANN-I model, as seen Fig2; one hidden layer was selected. In the hidden layer 10 neurons were determined due to its minimum absolute percentage error values for training and testing sets. In ANN-II model, as seen Fig. 3; two hidden layers were selected. In the first hidden layer ten neurons and in the second hidden layer ten neurons were determined due to its minimum absolute percentage error values for training and testing sets. In the Fig 4 and 5 shows same process as first set of inputs, here metakaolin used instead of silica fume.

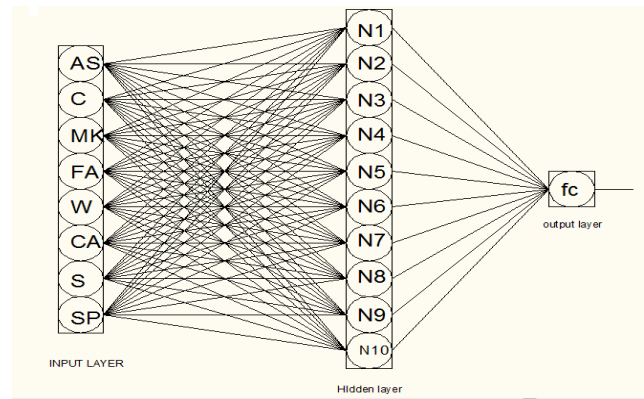


Fig 4. The system used in the ANN-II model

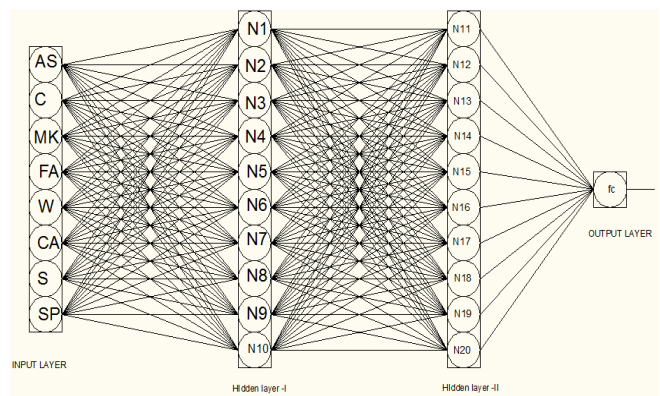


Fig 5. The system used in the ANN-II model

The limit values of input and output variables used in ANN-I and ANN-II models of cubes are listed in below Tables

Table3: The input and output quantities used in ANN models for concrete cubes

Input variables	Data used in training and testing the models	
	Minimum	Maximum
Age of specimen (day)	3	90
Cement (kg/m <sup>3</sup> )	457.53	571.91
Silica fume (kg/m <sup>3</sup> )&Metakaolin	0	57.16
Fly ash (kg/m <sup>3</sup> )	0	57.16
Water (l)	171.47	171.47
Sand (kg/m <sup>3</sup> )	566.82	609.72
Aggregate (kg/m <sup>3</sup> )	1171.80	1171.80
Super plasticizer(l/m <sup>3</sup> )	6.97	17.19
Output variable		
Compressive strength(MPa)for SF	30.8534	75.8197
Compressive strength for Mk	30.6992	79.7649

Table4: The values of parameters used in models for concrete cubes

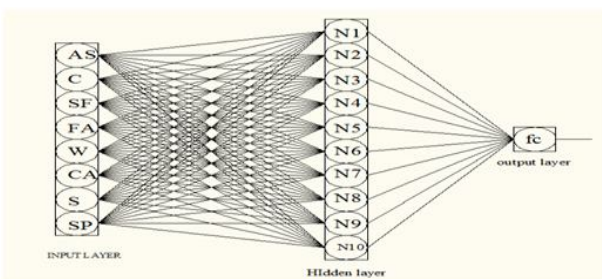


Fig 2. The System Used In the ANN-I Model

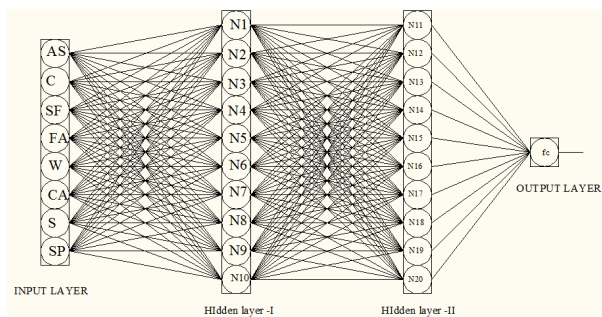


Fig 3. The system used in the ANN-II model.

Parameters	ANN-I	ANN-II
Number of input layer neurons	8	8
Number of hidden layer	1	2
Number of first hidden layer neurons	10	10
Number of second hidden layer neurons	10	10
Number of output layer neurons	1	1
Momentum rate	0.7	0.7
Learning rate	0.3	0.3
Error after learning	0.00100	0.000125
Learning cycle	5000	5000

### III. RESULTS AND DISCUSSION

#### FORMULATION

In this study, the error arose during the training and testing in ANN-I and ANN-II models can be expressed as a root-mean squared (RMS) error and is calculated by Eq. (6.0).

$$RMS = \sqrt{\frac{1}{p} \sum_i |t_i - o_i|^2}$$

In addition, the absolute fraction of variance ( $R^2$ ) and mean absolute percentage error (MAPE) are calculated by Esq. (6.1) and (6.2), respectively.

$$R^2 = 1 - \left( \frac{\sum_i (t_i - o_i)^2}{\sum_i (o_i)^2} \right),$$

$$MAPE = \left| \left( \frac{t_i - o_i}{o_i} \right) \right| * 100$$

Here  $t$  is the target value,  $o$  is the output value,  $p$  is the pattern. In the training and testing of ANN-I and ANN-II models, various experimental data from two different sources are used. In the ANN-I and ANN-II models, 140 data of experiment results were used for training whereas 65 ones were employed for testing.

The predicted high performance concrete results of ANN-I and ANN-II for compressive strength of cubes, also input values and experimental results with testing results obtained from ANN-I and ANN-II models. In the below figures shows the comparison of experimental results with predicted results of ANN.

Table5: Testing data sets for comparison of experimental results with testing results predicted from models (0.3w/b)

Data used in models construction								Compressive strength (MPa)			
AS(day)	C (kg/m <sup>3</sup> )	SF (kg/m <sup>3</sup> )	FA (kg/m <sup>3</sup> )	W (kg/m <sup>3</sup> )	CA (kg/m <sup>3</sup> )	S (kg/m <sup>3</sup> )	SP (l/m <sup>3</sup> )	Experimental results	ANN-I	ANN-II	% Error
3	571.19	0	0	171.47	1171.8	609.72	6.97	35.67	37.1806	35.6700	-4.23493
3	543.31	28.58	0	171.47	1171.8	599.81	8.83	32	35.1736	32.0000	-9.91438
3	529.01	42.87	0	171.47	1171.8	594.58	9.23	34.33	36.5486	38.1858	-6.46257
3	514.72	57.16	0	171.47	1171.8	589.35	9.75	32.67	32.3999	32.6700	0.826752
3	486.12	28.58	57.16	171.47	1171.8	577.28	16.72	33.67	33.1987	33.6700	1.399762
3	471.82	42.87	57.16	171.47	1171.8	572.05	17.19	31.33	32.7407	31.3300	-4.50271
3	457.53	57.16	57.16	171.47	1171.8	566.82	17.19	29	30.6082	29.0000	-3.53862
7	571.19	0	0	171.47	1171.8	609.72	6.97	42.33	40.5497	42.3300	4.205764
7	543.31	28.58	0	171.47	1171.8	599.81	8.83	40.67	39.9614	38.0635	-1.742316
7	529.01	42.87	0	171.47	1171.8	594.58	9.23	44.67	42.1719	44.6700	5.592344
7	514.72	57.16	0	171.47	1171.8	589.35	9.75	40.33	38.2956	38.7379	5.044384
7	486.12	28.58	57.16	171.47	1171.8	577.28	16.72	42.33	38.2867	42.3300	9.551854
7	471.82	42.87	57.16	171.47	1171.8	572.05	17.19	41	38.2146	40.7288	6.793659
7	457.53	57.16	57.16	171.47	1171.8	566.82	17.19	39.33	36.1621	39.3300	8.054666
28	571.19	0	0	171.47	1171.8	609.72	6.97	54.67	55.3674	54.6700	-1.27565
28	543.31	28.58	0	171.47	1171.8	599.81	8.83	55	57.9681	55.0000	-5.39655
28	529.01	42.87	0	171.47	1171.8	594.58	9.23	61.33	61.8237	61.3300	-0.80499
28	514.72	57.16	0	171.47	1171.8	589.35	9.75	56.33	55.8945	54.2256	-0.773123
28	486.12	28.58	57.16	171.47	1171.8	577.28	16.72	58.67	59.8284	58.6700	-1.96992
28	471.82	42.87	57.16	171.47	1171.8	572.05	17.19	57.33	58.0191	57.3300	-1.20199
28	457.53	57.16	57.16	171.47	1171.8	566.82	17.19	55.33	55.0602	55.3300	0.48762
56	571.19	0	0	171.47	1171.8	609.72	6.97	61.33	61.2641	61.3300	0.107451
56	543.31	28.58	0	171.47	1171.8	599.81	8.83	61.67	62.4786	67.8248	-1.31117
56	529.01	42.87	0	171.47	1171.8	594.58	9.23	69.67	68.4972	69.6700	1.683364
56	514.72	57.16	0	171.47	1171.8	589.35	9.75	65.67	65.6221	62.0506	0.07294
56	486.12	28.58	57.16	171.47	1171.8	577.28	16.72	68.67	67.1433	68.6700	2.223242
56	471.82	42.87	57.16	171.47	1171.8	572.05	17.19	68.33	61.2353	67.5019	10.38299
56	457.53	57.16	57.16	171.47	1171.8	566.82	17.19	60.33	60.8411	60.3300	-0.84717
90	571.19	0	0	171.47	1171.8	609.72	6.97	66.67	66.5322	66.6700	0.20669
90	543.31	28.58	0	171.47	1171.8	599.81	8.83	67.67	67.9173	67.6700	-0.36545
90	529.01	42.87	0	171.47	1171.8	594.58	9.23	76.33	71.4487	76.3300	6.394985
90	514.72	57.16	0	171.47	1171.8	589.35	9.75	71.67	72.0549	71.6700	-0.53704
90	486.12	28.58	57.16	171.47	1171.8	577.28	16.72	74.33	74.3562	78.7473	-0.03525
90	471.82	42.87	57.16	171.47	1171.8	572.05	17.19	73.67	73.4628	76.8461	0.281254
90	457.53	57.16	57.16	171.47	1171.8	566.82	17.19	67.33	75.5732	67.3300	-12.243

Table 6: Testing data sets for comparison of experimental results with testing results predicted from models (0.3w/b)

Table with 13 columns: AS(day), C, MK, FA, W, CA, S, SP, Experimental results, ANN-I, ANN-II, % Error. Rows include data for days 3, 7, 28 and 90 with various material properties and model results.

Table 7 Testing data sets for comparison of experimental results with testing results predicted from models (0.32w/b)

Table with 13 columns: AS (day), C, SF, FA, W, CA, S, SP, Experimental results, ANN-I, ANN-II, % Error. Rows include data for days 3, 7, 28 and 90 with various material properties and model results.

Table with 13 columns: AS, C, MK, FA, W, CA, S, SP, Experimental results, ANN-I, ANN-II, % Error. Rows include data for days 28, 56, and 90 with various material properties and model results.

Table 8 Testing data sets for comparison of experimental results with testing results predicted from models (0.32w/b)

Table with 13 columns: AS (day), C, MK, FA, W, CA, S, SP, Experimental results, ANN-I, ANN-II, % Error. Rows include data for days 3, 7, 28, and 90 with various material properties and model results.

7	457.53	57.16	57.16	182.90	1171.8	566.82	17.19	54.05	51.3431	47.8876	5.008141
28	571.19	0	0	182.90	1171.8	609.72	6.97	61	59.7119	56.6797	2.111639
28	543.31	28.58	0	182.90	1171.8	599.81	8.83	63.7	65.1722	60.2211	-2.31115
28	529.01	42.87	0	182.90	1171.8	594.58	9.23	67	66.5070	65.4213	0.735821
28	514.72	57.16	0	182.90	1171.8	589.35	9.75	65.2	62.2260	63.3655	4.56135
28	486.12	28.58	57.16	182.90	1171.8	577.28	16.72	66	59.9868	67.1228	9.110909
28	471.82	42.87	57.16	182.90	1171.8	572.05	17.19	68.5	67.3223	65.6470	1.71927
28	457.53	57.16	57.16	182.90	1171.8	566.82	17.19	64.8	64.5692	59.1054	0.356173
56	571.19	0	0	182.90	1171.8	609.72	6.97	66	66.5495	67.4413	-0.83258
56	543.31	28.58	0	182.90	1171.8	599.81	8.83	70	71.0091	68.4202	-1.44157
56	529.01	42.87	0	182.90	1171.8	594.58	9.23	72.95	74.2301	74.1683	-1.75476
56	514.72	57.16	0	182.90	1171.8	589.35	9.75	69	74.5866	72.0435	-8.09652
56	486.12	28.58	57.16	182.90	1171.8	577.28	16.72	70.42	71.5630	68.4062	-1.62312
56	471.82	42.87	57.16	182.90	1171.8	572.05	17.19	74	72.5775	70.8677	1.922297
56	457.53	57.16	57.16	182.90	1171.8	566.82	17.19	71.5	71.9113	71.9234	-0.57524
90	571.19	0	0	182.90	1171.8	609.72	6.97	66.67	66.7330	66.4152	-0.0945
90	543.31	28.58	0	182.90	1171.8	599.81	8.83	72.33	70.4433	69.8030	2.608461
90	529.01	42.87	0	182.90	1171.8	594.58	9.23	77.33	71.7324	74.5534	7.238588
90	514.72	57.16	0	182.90	1171.8	589.35	9.75	71	72.5326	72.0904	-2.15859
90	486.12	28.58	57.16	182.90	1171.8	577.28	16.72	73.33	74.8022	73.2528	-2.00764
90	471.82	42.87	57.16	182.90	1171.8	572.05	17.19	80.67	79.2472	67.9087	1.763729
90	457.53	57.16	57.16	182.90	1171.8	566.82	17.19	72.33	85.9765	67.8394	-18.867

All results, obtained from experimental studies and predicted by using the training and testing results of ANN I and ANN II models, for 3, 7, 28, 56, and 90 days  $f_c$  were given in Figs. 6 and 7, respectively. The testing results obtained from ANN-I and ANN-II models were given in Tables. As it is visible in Figs. 6 and 7. The values obtained from the training and testing in ANN-I and ANN-II models are very closer to the experimental results.

The performance of the ANN models for compressive strength of cubes  $f_c$ , is shown in Figs. 6 to 7, respectively. The statistical values for all the station such as RMS,  $R^2$  and MAPE, both training and testing, are given in Table 9 While the statistical values of RMS,  $R^2$  and MAPE from training in the ANN-I model were found as 2.1422, 99.12% and 1.8514%, respectively, these values were found in testing as 2.2551, 99.01% and 0.4287%, respectively. Similarly, while the statistical values of RMS,  $R^2$  and MAPE from training in the ANN-II model were found as 4.4043, 99.65% and 3.7135%, respectively, these values were found in testing as 4.7382, 94.99% and 3.3920%, respectively. The best value of  $R^2$  is 99.65% for training set in the ANN-II model. The minimum value of  $R^2$  is 99.01% for testing set in the ANN-I model. All of the statistical values in Table 9 show that the proposed ANN-I and ANN-II models are suitable and predict the 3, 7, 28, 56, and 90 days  $f_c$  values very close to the experimental values.

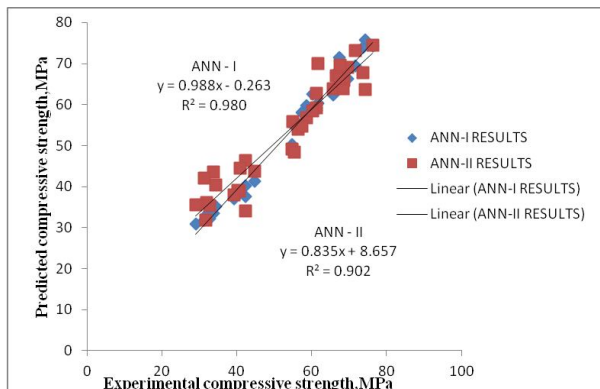


Fig. 6. Comparison of  $f_c$  experimental results with predicted results

Table.9. The  $f_c$  statistical values of proposed ANN-I and ANN-II models for SF&FA(0.3w/b)

Statistical parameters	ANN-I		ANN-II	
	Training set	Testing set	Training set	Testing set
RMS	2.1422	2.2551	4.4043	4.7382
$R^2$	0.9912	0.9901	0.9965	0.9459
MAPE(%)	1.8514	0.4287	3.7135	3.3920

IV. CONCLUSION

Artificial neural networks are capable of learning and generalizing from examples and experiences. This makes artificial neural networks a powerful tool for solving some of the complicated civil engineering problems. In this study, using these beneficial properties of artificial neural networks in order to predict the 3, 7, 14, 28, 56, and 90 compressive strength of concretes containing metakaolin and silica fume along with fly ash without attempting any experiments were developed with two different multilayer artificial neural network architectures namely ANN-I and ANN-II. In the two models developed in ANN method, a multilayered feed forward neural network with a back propagation algorithm

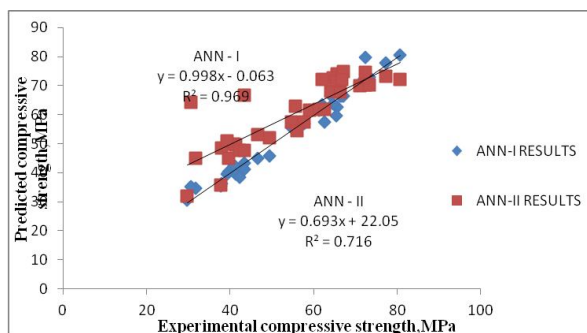


Fig..7. Comparison of  $f_c$  experimental results with predicted results



was used. In ANN-I model, one hidden layer was selected. In the hidden layer 10 neurons were determined.

In ANN-II model, two hidden layers were selected. As a result, compressive strength of concretes containing metakaolin and silica fume can be predicted in the multilayer feed forward artificial neural networks models without attempting any experiments in a quite short period of time with tiny error rates.

The obtained conclusions have demonstrated that multilayer feed forward artificial neural networks are practicable methods for predicting compressive strength values of concretes containing metkaolin and silica fume.

### REFERENCES

- [1] Metha and Monterio, "Concrete: micro structure, properties and materials," Indian edition, Indian concrete institute, Chennai, 1999.
- [2] Nawy, "Fundamentals of high performance concrete," Second edition, John Wiley and Sons Inc., New York, 2001.
- [3] Shah and Ahmad, High performance concretes and applications, Edward Arnold, London, 1994.
- [4] Joshi, "Evolution of HPC mixes containing silica fumes," The Indian concrete journal, vol. 75, no. 10, pp 627-633, 2001. 5. Rixom and M. Vaganam, Chemical admixtures for concrete, second edition, E& F.N Spon, London, 1996.
- [5] Basu, "NPP Containment structures: Indian experience in silica fumes based HPC," The Indian concrete journal, vol.75, no.10, pp. 656-664, October 2001.
- [6] Rathish Kumar P, 'High Performance Superplasticized Silica Fume Mortars for Ferrocement Works', Architecture and Civil Engineering, Vol. 8, pp. 129-134, 2010.
- [7] Vinayagam P, 'Experimental Investigation on HPC Using Silica Fume and Superplasticizer', International Journal of Computer and Communication Engineering, Vol. 1, pp. 2, 2012.
- [8] Magudeaswaran P. and Eswaramoorthi P, 'Experimental Study on Durability Characteristics of HPC', International Journal of Emerging Technology and Advanced Engineering, Vol. 3, pp. 2250-2259, 2013.
- [9] Nasratullah Amarkhail, 'Effect of Silica Fume on Properties of High Strength Concrete' International Journal of Technical Research and Applications, Issue 32, pp.13-19, 2015