

# Study on Stabilization of Materials In Pavement Sections

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**Abstract-** *Abstract In olden days, transportation of goods and services carried out through natural earth surface, that soil was compacted well by the humans and the movement of animals on roads due to this tracks were developed. These roads formation had happened with mud and sand. however these roads can sustain the medium traffic conditions but it will be worse on rainy conditions, later in 18 th century several Pavement design assumptions were developed stating that the necessity of aggregates in pavement sections. When comes to the present scenario several pavement techniques were developed .in order to bind the soil and aggregate we need some cementious material other than the cement .in this study several industrial waste is using as a cementious material by this we can protect the environment. By treating natural soil or fly-ash, or by addition of certain materials to it, new road construction materials can be developed. Thus use of stabilized soil/waste product as a base/sub-base material of pavement leads not only economic solution, but also offers a potential use of the industrial/domestic waste materials. Thus cemented bases and sub-bases can be designed to economize the design where, locally available inferior quality materials are stabilized using cemented material. In this research work, it is proposed to investigate the best combination of soil-cement-water and fly-ash-lime-water-sodium chloride-fibre-bitumen for the purpose of deciding its performance as pavement base/sub-base material. Fly-ash is stabilized by adding water, lime, sodium chloride, fibres, and bitumen. Combination of soil-cement in the presence of water is considered as another material for pavement base or sub-base*

## I. NEED OF STABILIZATION

Aggregates Are The Main Important Thing In The Pavement Design, The Good Quality Of Aggregates Is Essential For Quality Pavement Design, Generally At The Construction Site Locally Available Materials Were Used Because To Avoid The Transportation Charges And It Also A Time Taking Process. All the Locally Available Materials Are Not Such Good that to Exhibit The Engineering Properties Of The Aggregates, so to develop such engineering properties stabilization of aggregates is required, the stabilization material which is used in this study is an industrial waste, in this regard we can easily find the solution for the industrial

and the stabilization will be in economical and the materials are

- Fly-ash-Lime-Water-NaCl-Fibre-Emulsion
- Soil-water-cement

## II.BASIC INGREDIENTS FOR CEMENTED BASE/SUB-BASE IN PAVEMENT

Fly-ash, Portland cement and lime are most commonly used cementing materials for construction of cemented base with soil, sand and aggregates for pavements of roads and runways. Low grade or marginal aggregates with suitable proportioning of coarse and fine fractions, lime clay, lime-Laterite-soil, lime-fly-ash, lime-granulated blast-furnace-slag-soil mixture, etc. can be used as a cemented base or sub-base. Various types of materials are considered by several researchers to study the suitability of those materials as pavement base and sub-base layer. Natural soil, fly-ash, sand, stone dust, river bed materials, reclaimed asphalt pavement, low quality aggregates etc. are considered as ingredients of pavements with cemented base/sub base. Cemented materials may be classified in to three categories:

1. Traditional stabilizers: hydrated lime, Portland cement, and fly-ash
2. By-product stabilizers: cement kiln dust, lime kiln dust and other forms of by product lime
3. Non-traditional stabilizers: sulfonated oils, potassium compounds, ammonium chloride,

Enzymes, polymers

## III. MIX DESIGN

Mix design is the process of choosing the optimum stabilized content of various ingredients of the pavement. The general principle of mix design is that the mixture should provide satisfactory performance when constructed in the desired position in the pavement structure of sub-grade. Design proportions of the ingredients are generally based on an analysis of the effect of various proportions on selected engineering properties of the mix. Numerous research publications and technical guides are available to aid the

engineer in the selection criteria to determine the amount of each ingredient. A wide variety of test methods have been proposed such as- California procedure, Eades and Grim procedure, Illinois procedure, Louisiana procedure, Oklahoma procedure, South dekota procedure, Texas procedure, Thompson procedure, Virginia procedure etc. Engineering properties which are considered, depending on the objective are- Atterberg limits California Bearing Ratio (CBR), swell potential, unconfined compression strength (UCS) of cured or uncured mixtures, Freeze-thaw and wet-dry test etc. The mix design procedure includes the testing for strength and for durability. Most researchers reported that a minimum of 3 percentage lime is necessary to produce adequate reaction in the field. The National Lime Association recommends 3, 5 and 7 percentage lime in the trial mixes.

#### IV. PAVEMENT DESIGN

Mechanistic-empirical pavement design procedure has become popular in many countries including USA, Australia, South Africa and other countries in Europe. Though the design parameters viz., traffic and environment may vary from country to country, the basic principles of analytical design are universal in nature. Indian Road Congress suggests some guidelines which deal with some practical issues related to the use of cemented materials in pavement structure, and are not applicable for structural design of bituminous pavement with cemented base or sub-base. Das and Pandey proposed methodology for mechanistic design of pavement with cemented base which is similar to that proposed by Austroads, South African Code, and other researchers. Singh and Baffour proposed the design methodology for lime stabilized Laterite roads, in which he assumed two approaches as cracked and no cracking approach. Cracked approach makes use of an assumption that the pavement possesses slab-type behaviour. In this approach, selection of the base thickness is made on the basis of flexural stress of the stabilized layer, the tensile strains in both the asphalt and the stabilized layer and the vertical compressive strain at the top of sub grade. The second approach (no cracking) makes the assumption that the lime stabilized layer will crack shortly after construction, hence it can be assumed as well-compacted granular material. Due to cracking the modulus of stabilized layer will reduce. Otte et. al. proposed the mechanistic structural design procedure for pavements with cement and lime treated layers, in which he considered the traffic associated cracking as limiting distress condition. The gradual cracks in cemented material results in reduction in the effective elastic modulus value. AASHTO guidelines also suggest that an additional distress that needs to be considered in cemented pavements is fatigue fracture in the underlying chemically stabilized base layers. Under repeated applications

of loading, micro cracks form in these layers leading to a stiffness or modulus reduction and ultimately to fatigue fracture. This process will have a impact on the distress progression in the overlying layers, which will reduce the support provided to the upper pavement layers. AASHTO also suggests the performance criteria for fatigue cracking in chemically stabilized which is defined in terms of a damage index. Typical design damage index values for fatigue cracking in chemically stabilized layers are on the order of 25 %.

1. The first phase is the period soon after construction, when the elastic modulus of cemented layer is high. Hence original elastic modulus (E) value is taken as initial E value of the cemented material. The cracks get propagated due to the shrinkage, thermal effects and construction traffic. Thus the effective elastic modulus drops from E1 to E2 soon after the pavement is opened to traffic. As phase I is too short, it is not considered within the design period

2. For phase II, tensile strain is computed at the bottom of the cemented base with elastic modulus as E2. The fatigue equation of the material can be used to calculate the fatigue life of the cemented base. Since modulus of the bituminous layer is lower than cemented layer, flexural fatigue of bituminous surfacing will not govern the pavement performance as long as cemented layer is intact.

3. When the life of cemented material is consumed completely, it becomes fully cracked and no longer behaves like a layered elastic material, rather it can be considered as a granular layer with an elastic modulus of E3, for the phase III. The elastic modulus will reduce due to cracking and several guidelines suggest various percentage reductions. For example, South African guidelines suggest 90 % reduction in elastic modulus. The fatigue life for the third phase is estimated from the maximum tensile strain at the bottom of bituminous layer.

4. The layer thickness of bituminous layer and cemented layers are adjusted such a way that the sum of these two fatigue lives becomes the design life.

#### V. SUMMARY

The summary of the present study is presented below.

1. Various ingredients materials and various mix design procedures for cemented/stabilized layer in the pavement structure has been reviewed.

2. Two types of mixes are considered in the present study viz,

(i) FLWSRE mix (fly-ash + lime + water + sodium chloride + fibre + bitumen) and

(ii) SCW mix (soil + cement + water).

The optimal proportion of the ingredients of the mix is obtained by performing UCS test.

3. CBR test is performed to check the suitability of the mix as a pavement material, and result of soaked and unsoaked CBR test indicates that the proposed proportion of the ingredients can be used as pavement base and sub-base layer.

4. The elastic modulus and fatigue life of the SCW mix is evaluated by performing fatigue test on the SCW mix. Fatigue curve is developed for SCW mix and attempt has been made to explain cemented pavement design procedure with an example.

## VI.CONCLUSION

1. The optimal proportion of the ingredients of FLWSRE mix is obtained as, 33 % water, 11 % lime, 0.30 % 12 mm fibers, 1 % sodium chloride, and 1.3 % bitumen which gives maximum UCS of the order of 390 kPa.

2. The optimum proportions of ingredients of SCW mix is obtained as 6 % cement content and 14 % water content which gives the maximum UCS of order of 1.388 MPa.

3. The unsoaked and soaked CBR of FLWSRE mix is obtained 23.26 % and 18.96 % respectively.

4. The unsoaked and soaked CBR of SCW mix is obtained 86.16 % and 73.31 % respectively.

5. From UCS, CBR and fatigue test results on SCW mix, following relationship can be developed for elastic modulus.  $E = 0.928 \times \text{CBR}$  and  $E = 57.64 \times \text{UCS}$

6. From fatigue tests on SCW mix, elastic modulus of mix is obtained. From fatigue life Vs strain plot, design of bituminous pavement with cemented base is performed and compared with conventional design of bituminous pavement with granular base. It has been observed that the life of former is more than that of later configuration. Hence bituminous pavement with cemented base is economical than bituminous pavement with granular base.

## FUTURE SCOPE

This section gives some of the works that can be taken as further study-

1. Various combinations of the materials, like recycled aggregates, poor quality aggregates etc. can be tried and the mix which gives maximum fatigue life can be obtained.

2. Most economical combination of the layer thickness and material properties can be obtained by performing economical analysis.

3. Durability test can be performed to ensure about the performance of stabilized layer.

4. Rutting equation can be developed for bituminous pavement with cemented base.

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