

Seismic Analysis of RC Structure Using Different Country Codes

Ritika Pathak¹, Savita Maru²

¹Dept of Civil Engineering

²Professor, Dept of Civil Engineering

^{1,2}Ujjain Engineering College, Ujjain, M.P., India

Abstract- This study explores the seismic performance of reinforced concrete (RC) structures using a comparative approach to evaluate three major seismic design codes: IS 1893:2016 (India), GB 50011-2010 (China), and the Turkish Earthquake Code 2018. A 60-meter-high RC building was modeled using STAAD.Pro software to assess structural behavior under seismic loads. Key performance parameters, including base shear, nodal deformation, and plate stress distribution, were analyzed. The findings highlight significant variations in design philosophies, with the Indian code demonstrating moderate deformation control, the Chinese code adopting a conservative approach, and the Turkish code emphasizing ductility and strength balance. These differences impact structural resilience and provide insights into enhancing global seismic safety standards. This study contributes to the understanding of regional seismic design practices and proposes recommendations for harmonizing international seismic codes to mitigate earthquake risks effectively. The results aim to advance engineering strategies for earthquake-prone regions.

Keywords- Seismic analysis, RC structures, STAAD.Pro, IS 1893:2016, GB 50011-2010, Turkish Earthquake Code, comparative study.

I. INTRODUCTION

The increasing urbanization and the consequent rise of high-rise structures have significantly amplified the need for robust seismic design practices, especially in earthquake-prone regions. High-rise buildings, while architecturally and economically significant, pose unique challenges in terms of their seismic performance due to their height, mass distribution, and structural complexities. To mitigate the risks of seismic damage, countries adopt seismic design codes tailored to their regional seismicity, geological conditions, and construction practices. These codes, such as IS 1893:2016 in India, GB 50011-2010 in China, and the Turkish Earthquake Code 2018, play a crucial role in standardizing structural safety measures by specifying design spectra, load combinations, and detailing requirements. Comparing these codes helps uncover differences in design philosophies and

evaluate their effectiveness in ensuring resilience in high-rise structures (Paul, Saha, & Dutta, 2017; Erdik, 2019).

Despite significant advancements in seismic engineering, global disparities in seismic design practices persist, often shaped by regional hazards and construction technologies. For instance, while the Indian code emphasizes cost-effectiveness with moderate deformation control, the Chinese code reflects a conservative approach with higher base shear values, and the Turkish code prioritizes ductility for energy dissipation. These variations highlight the necessity of harmonizing seismic standards to improve structural resilience and safety globally. This study employs STAAD.Pro software to perform a comparative seismic analysis of a 60-meter RC building, focusing on key performance metrics such as base shear, nodal displacement, and plate stress distribution. The findings aim to provide actionable insights for refining seismic design codes, ensuring the safety and sustainability of high-rise buildings in seismic zones (Chopra et al., 2017; Gülerce & Rezazadeh, 2016).

II. LITERATURE REVIEW

Recent studies in seismic performance assessment emphasize the critical role of advanced modeling techniques and country-specific seismic codes in ensuring the safety of reinforced concrete (RC) structures. Kapoor and Gupta (2021) used STAAD.Pro to analyze the seismic responses of tall buildings as per Indian, Chinese, and Turkish codes, highlighting significant differences in base shear and lateral drift. Similarly, Kumar and Prasad (2020) examined the variations in seismic load assumptions across these codes, revealing how regional design philosophies influence structural behavior. These studies underline the necessity of adapting design strategies to regional seismicity while leveraging computational tools to improve analysis accuracy. Contemporary research also focuses on performance-based seismic design and energy dissipation mechanisms. Erdogan et al. (2019) discussed the Turkish Earthquake Code 2018's emphasis on ductility and the integration of energy dissipation devices for enhanced resilience. Zhou et al. (2020) evaluated China's GB 50011-2010, highlighting its probabilistic hazard

approach and superior site-specific response spectra. Additionally, Liu et al. (2020) explored the applicability of advanced computational methods in refining seismic analysis under Chinese and international codes. These findings reflect the ongoing advancements in seismic engineering and the growing need for harmonized global standards to address the unique challenges posed by high-rise structures in earthquake-prone regions.

III. METHODOLOGY

3.1 General

This chapter describes the systematic approach adopted to compare the seismic performance of RC structures based on Indian, Chinese, and Turkish seismic codes using STAAD.Pro software. The methodology encompasses structural modelling, load definition, seismic analysis, and data interpretation to evaluate critical parameters such as base shear, story drift, and displacement. The study focuses on understanding how tall buildings respond to seismic forces under different design philosophies.

The analysis centres on a 60-meter-tall high-rise building with dimensions of 15m x 12m and a story height of 3m. The building features RCC columns (0.35m x 0.30m), RCC beams (0.35m x 0.30m), and a slab thickness of 0.18m. Dead loads were calculated based on IS 875 Part 1:1987, while live loads were derived from IS 875 Part 2:1987. Seismic loads were incorporated according to the Indian (IS 1893:2016), Chinese (GB 50011-2010), and Turkish seismic codes, ensuring a comprehensive comparison.

3.2 Plan of the Building



3.3 Structure Properties

The study analysed RC frames with the following configurations:

- **Bay Dimensions:** 15m (X-direction), 12m (Y-direction)
- **Total Heights:**
 - G+5: 15m
 - G+10: 30m

- G+15: 45m
- G+20: 60m

3.4 Member Properties

Table 1 provides details of the structural member dimensions, consistent across all configurations:

Table 1. Structural Member Dimensions

Member	Dimensions (meters)
Beam	0.35 x 0.30
Column	0.35 x 0.30
Slab	0.18

3.5 Load Considerations and Combinations

- **Dead Load (DL):** Self-weight with a factor of 1.5; member weight is 5 kN/m.
- **Live Load (LL):** Floor weight of 3 kN/m².
- **Seismic Loads:** Parameters were derived from the respective codes.

3.6 Assumptions

- The structure is analysed as per IS 1893:2016, GB 50011-2010, and TÜRKIYE DEPREM YÖNETMELII.
- Models assume uniform, isotropic, and linearly elastic materials.
- Columns are fixed at the foundation.
- Floors are rigid in the horizontal plane.

3.7 Structural Analysis

The models were developed and analysed in STAAD.Pro for all configurations (G+5, G+10, G+15, G+20). Seismic loads from the Indian, Chinese, and Turkish codes were applied to assess critical parameters such as base shear, story drift, and displacement.

3.8 Input Parameter Comparison

Key seismic parameters compared across the codes are summarized in Table 2:

Table 2. Input Parameter Comparison

Parameter	IS 1893:2016	GB 50011:2010	Turkish Code
Zone Factor	0.36 (Zone V)	PGA = 0.4g (Zone 9)	PGA > 0.3g
Response	2.5	2–3 (Behaviour)	2–3

Reduction Factor		Factor)	
Soil Type	Medium	Site Class 2 or 3	Site Class 2 or 3
Damping Ratio	5%	~0.05 (Equivalent)	5%
Period (X/Z) (Sec)	0.5	0.5	0.5

3.9 Results Overview

Critical results obtained through the analysis include:

- **Base Shear:** Comparative values reflecting the varying seismic intensity assumptions.
- **Nodal Displacement:** Maximum displacement values highlight the flexibility and resilience of structures.
- **Plate Stresses:** Stress distribution in X and Z directions provides insights into structural integrity.

This structured approach ensures the accuracy and relevance of the findings, offering a basis for evaluating the seismic resilience of RC structures under different international codes.

IV. RESULTS AND DISCUSSION

Table 2 shows the results obtained from analysis;

Table 2. Comparative Analysis of Results

Parameter	Indian Code (IS 1893:2016)	Chinese Code (GB 50011-2010)	Turkish Code (Turkish Earthquake Code 2018)
Deformation Trend	G+5: 67.230 mm, G+10: 594.840 mm, G+15: 1379.64 mm, G+20: 2664.889 mm.	G+5: 184.220 mm, G+10: 680.405 mm, G+15: 1618.875 mm, G+20: 3156.270 mm.	G+5: 172.070 mm, G+10: 475.142 mm, G+15: 971.579 mm, G+20: 1920.443 mm.
Plate Stress (MPa)	X: G+5 = 0.836, G+10 = 3.95, G+15 = 5.86, G+20 = 7.8; Z: G+5 = 0.906,	X: G+5 = 1.9, G+10 = 3.59, G+15 = 5.34, G+20 = 6.34; Z: G+5 = 2.08, G+10 = 3.96, G+15 = 5.93, G+20 = 8.17.	X: G+5 = 2.86, G+10 = 4.17, G+15 = 5.55, G+20 = 7.36; Z: G+5 = 1.63, G+10 = 2.39, G+15 = 3.05, G+20 = 4.12.

	G+10 = 4.34, G+15 = 6.45, G+20 = 8.58.		
Base Shear (G+20)	Fx = 1121.68 kN, Fy = 16402.37 kN, Fz = Not specified.	Fx = Not specified, Fy = 18542.98 kN, Fz = Not specified.	Fx = 1133.98 kN, Fy = 10662.36 kN, Fz = Not specified.
Overturning Moment	G+5: 206.649 kN-m, G+10: 809.152 kN-m, G+15: 1371.322 kN-m, G+20: 1850.482 kN-m.	G+5: 167.850 kN-m, G+10: 647.255 kN-m, G+15: 1210.628 kN-m, G+20: 1571.822 kN-m.	G+5: 210.420 kN-m, G+10: 725.341 kN-m, G+15: 1394.203 kN-m, G+20: 1869.585 kN-m.

Overview of Deformation Trends

The total nodal deformation of a structure under seismic loading is a critical metric for assessing its flexibility and resilience during earthquakes. Deformation values for buildings of different heights (G+5, G+10, G+15, and G+20) under Indian, Chinese, and Turkish seismic codes provide insights into the seismic design philosophies adopted by each country.

The Indian code demonstrates the lowest total nodal deformation across all building heights. The deformation increases steadily with building height, from 67.230 mm for G+5 to 2664.889 mm for G+20. This trend reflects a more conservative approach to deformation control, aiming for structural stability and minimal displacement during seismic events. The lower deformation values suggest stricter criteria for structural stiffness and seismic safety.

The Chinese code exhibits the highest deformation values among the three codes, with a range from 184.220 mm for G+5 to 3156.270 mm for G+20. These values reflect higher assumptions of seismic intensity or more flexible structural designs to accommodate energy dissipation during earthquakes. The significant increase in deformation with height indicates a design philosophy that prioritizes flexibility over rigidity.

The Turkish code presents moderate deformation values, ranging from 172.070 mm for G+5 to 1920.443 mm for G+20. This balanced approach emphasizes a compromise between structural flexibility and safety. The Turkish code aims to provide adequate deformation capacity to prevent structural failure while maintaining resilience.

The deformation trends highlight distinct seismic design philosophies. The Indian code emphasizes minimal deformation for enhanced stability, the Chinese code allows higher flexibility to absorb seismic energy, and the Turkish code adopts a balanced approach. These variations impact the overall building resilience, with each code addressing regional seismic hazards and structural requirements.

PLATE STRESS ANALYSIS

The stress distribution in the X and Z directions for buildings of varying heights under Indian, Chinese, and Turkish seismic codes is a critical parameter for understanding load distribution and structural response under seismic forces.

The plate stress values for the Indian code increase progressively with building height. The X direction stresses rise from 0.836 MPa for G+5 to 7.8 MPa for G+20, while Z direction stresses increase from 0.906 MPa to 8.58 MPa. This indicates a well-distributed load transfer mechanism, ensuring structural integrity under seismic loads.

The Chinese code exhibits slightly higher stresses at lower building heights. For G+5, the stresses are 1.9 MPa (X direction) and 2.08 MPa (Z direction), increasing to 6.34 MPa and 8.17 MPa for G+20, respectively. The stress distribution reflects a design philosophy that emphasizes base-level strength for seismic resistance.

The Turkish code demonstrates higher X direction stresses at lower building heights, starting at 2.86 MPa for G+5 and peaking at 7.36 MPa for G+20. The Z direction stresses are comparatively moderate, ranging from 1.63 MPa to 4.12 MPa, highlighting localized stress concentration in critical structural components.

The Indian code focuses on balanced stress increments with height, ensuring overall stability. The Chinese code prioritizes base-level strength, while the Turkish code emphasizes resistance in the X direction with moderate Z direction values. These differences underscore the impact of national seismic codes on stress management strategies.

BASE SHEAR ANALYSIS

The base shear values for G+5 buildings reveal varying load distributions. The Indian code exhibits moderate values with 126.134 kN (Fx), 601.845 kN (Fy), and 127.466 kN (Fz). The Chinese code shows significantly higher values in Fy (1697.41 kN) and Fz (282.95 kN), reflecting stringent seismic safety measures. The Turkish code presents the highest Fx value (466.76 kN), emphasizing lateral force resistance.

For G+10 buildings, the Indian code records balanced values (583.236 kN in Fx and 4781.155 kN in Fy), highlighting vertical load resistance. The Chinese code displays higher values in Fy (5443.77 kN), while the Turkish code focuses on Fx with 668.26 kN but exhibits lower Fy (3765.41 kN).

The G+15 base shear values reflect similar trends. The Indian code emphasizes uniform load distribution, with 852.04 kN in Fx and 9746.80 kN in Fy. The Chinese code prioritizes horizontal forces, with the highest Fy value (11133.24 kN). The Turkish code shows moderate resistance with 779.92 kN in Fx and 5907.51 kN in Fy.

In G+20 buildings, the Indian code maintains robust values across all directions, including 1121.68 kN in Fx and 16402.37 kN in Fy. The Chinese code peaks at 18542.98 kN in Fy, while the Turkish code balances lateral and vertical stability with 1133.98 kN in Fx and 10662.36 kN in Fy.

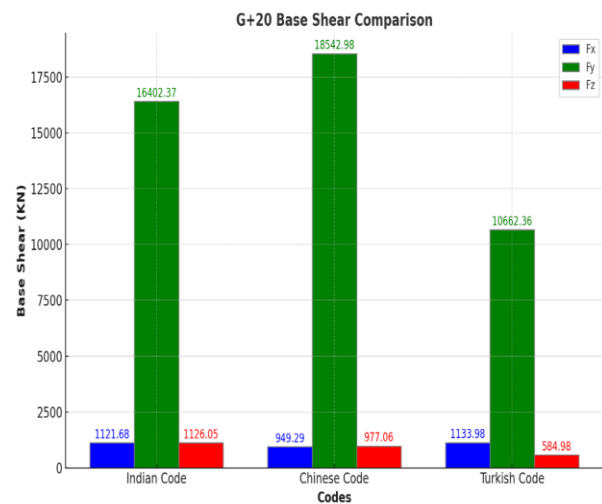


Figure 1. G+20 Base Shear Comparison

The Indian code adopts a conservative approach to distribute forces evenly, while the Chinese code prioritizes horizontal load resistance. The Turkish code emphasizes lateral forces, reflecting distinct regional design priorities.

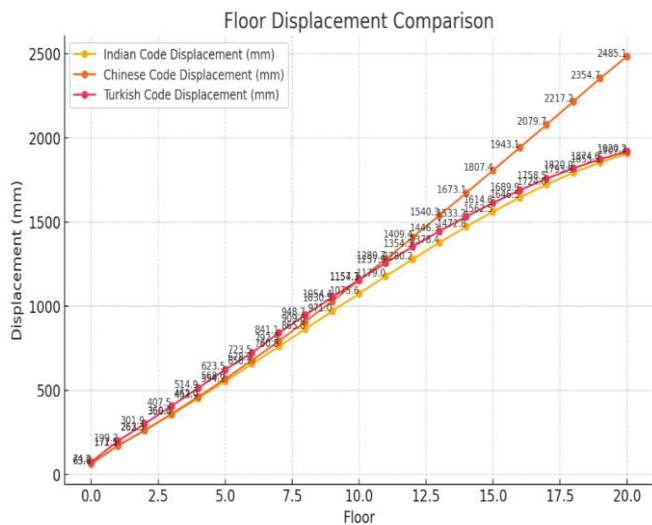


Figure 2. Displacement vs Floor for Different Codes (G+20)

OVERTURNING MOMENT ANALYSIS

The overturning moments for buildings of varying heights under Indian, Chinese, and Turkish codes highlight differences in structural behavior. The Indian code demonstrates steady increases, from 206.649 kN-m for G+5 to 1850.482 kN-m for G+20, reflecting its conservative design approach. The Chinese code presents moderate values, peaking at 1571.822 kN-m for G+20. The Turkish code shows higher moments at lower stories but converges at 1869.585 kN-m for G+20.

The Indian code ensures consistent safety with increasing height, while the Chinese code balances flexibility and stability. The Turkish code emphasizes resistance in lower stories, reflecting different design philosophies for overturning moment control in tall buildings.

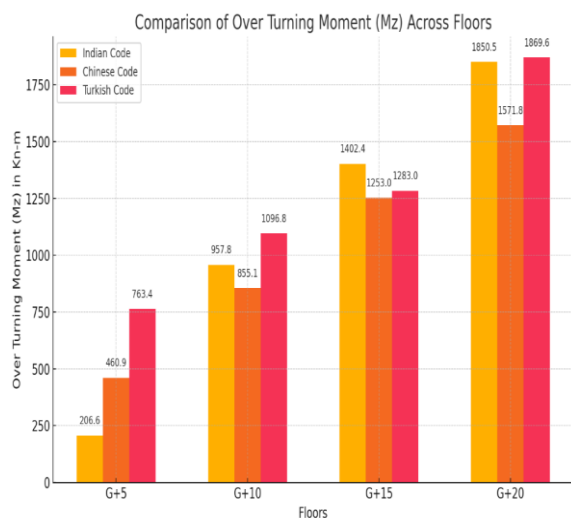


Figure 3. Comparison of Over Turning Moment (Mz) Across Floors

CONTRIBUTION TO SEISMIC ENGINEERING AND DESIGN PRACTICES

The findings from the displacement and drift analysis of G+20 structures under Indian, Chinese, and Turkish codes contribute significantly to the knowledge base of seismic engineering, particularly for regions prone to earthquakes. By analyzing and comparing the structural responses of tall buildings under these three international codes, this study provides critical insights into how varying seismic design philosophies affect the behavior of structures under seismic loads.

V. CONCLUSION

This study provides a comparative assessment of seismic performance in RC structures using Indian, Chinese, and Turkish seismic codes. Key findings highlight significant differences in design philosophies, reflecting diverse regional seismic priorities:

1. **Displacement Trends:**
 - The Indian code minimizes displacement (11%–62% less than others), emphasizing stability.
 - The Chinese code allows higher displacements (20%–50% more than the Indian code), promoting energy dissipation.
 - The Turkish code strikes a balance, achieving moderate displacements.
2. **Story Drift and Base Shear:**
 - Indian code restricts drift and base shear, enhancing structural safety.
 - Chinese code exhibits higher drift and base shear due to stringent seismic load considerations.
 - Turkish code demonstrates balanced drift and shear control, prioritizing ductility.
3. **Overturning Moments and Design Philosophy:**
 - Turkish code focuses on lower-story resistance, while Indian and Chinese codes ensure balanced overturning moment distribution.

The Indian code is conservative and accessible, ideal for basic seismic designs. The Chinese code is suited for regions with varied seismicity, while the Turkish code represents a modern, performance-based approach, setting a benchmark for advanced seismic practices.

REFERENCES

- [1] Sapkota A, Sapkota B, Poudel J, Giri S. Comparative study on the seismic performance of a typical low-rise building in Nepal using different seismic codes. *Asian J Civil Eng.* 2024. <https://doi.org/10.1007/s42107-024-01053-5>.
- [2] Kunwar, Sanjaya, Deepak Thapa, Achyut Paudel, and Aayush Shrestha. 2024. “Discover Civil Engineering A Comparative Analysis of an RC Low - Rise Building with the Seismic Codes of Countries Lying in the Himalayas : China , India , Nepal ,.” *Discover Civil Engineering*. doi: 10.1007/s44290-024-00122-7.
- [3] Kazemi, Farzin, Neda Asgarkhani, Torkan Shafighfard, Robert Jankowski, and Doo Yeol Yoo. 2024. *Machine-Learning Methods for Estimating Performance of Structural Concrete Members Reinforced with Fiber-Reinforced Polymers*. Springer Netherlands.
- [4] Ali, Abdihakim Osman. 2024. “Comparative Analysis of Seismic Design Standards for Structures and Safety Standards for High-Rise Concrete Building Structures.” *6(10):2–6*. doi: 10.53469/jpce.2024.06(10).01.
- [5] Bektaş, Nurullah, and Orsolya Kegyes-Brassai. 2024. *Developing a Machine Learning-Based Rapid Visual Screening Method for Seismic Assessment of Existing Buildings on a Case Study Data from the 2015 Gorkha, Nepal Earthquake*. Springer Netherlands.
- [6] Işık E, Avcil F, İzol R, Büyüksaraç A, Bilgin H, Harirchian E, Arkan E. Field reconnaissance and earthquake vulnerability of the RC buildings in adiyaman during 2023 Türkiye Earthquakes. *Appl Sci.* 2024;14:2860
- [7] Kunwar S, Thapa S. Adapting IS 1893 (2023) for the nepalese seismic context: evaluating base shear coefficients. *Int J Innov Res Eng Manage.* 2024;11(4):100–111. <https://doi.org/10.55524/ijirem.2024.11.4.13>
- [8] Shareef, Sardar S. 2023. “Earthquake Consideration in Architectural Design: Guidelines for Architects.” *Sustainability (Switzerland)* 15(18). doi: 10.3390/su151813760.
- [9] Ivanov ML, Chow WK, Structural damage observed in reinforced concrete buildings in Adiyaman during the 2023 Türkiye Kahramanmaraş Earthquakes. *Structures*, 2023;58.
- [10] STAAD.Pro. (2023). *STAAD.Pro user manual: Structural analysis and design software*. Bentley Systems.
- [11] Adhikari D, Adhikari S, Thapa D. A comparative study on seismic analysis of national building code of Nepal, India, Bangladesh and China. *Open Access Lib J.* 2022;9(e8933):1–11.
- [12] Bilgin H, Hadzima-Nyarko M, Isik E, Ozmen H, et al. A comparative study on the seismic provisions of different codes for RC buildings. *Struct Eng Mech.* 2022;83(2):195–206.
- [13] Tiwari, Dherendra, and Rakeh Patel. 2022. “Analysis of A Tall Structure Considering Two Different Type of Dampers Using Analysis Tool.” *6(3)*.
- [14] Liu C, Fang D, Zhao L. Reflection on earthquake damage of buildings in 2015 Nepal earthquake and seismic measures for post-earthquake reconstruction. *Structures.* 2021;30:647–58.
- [15] Shrestha JK, Paudel N, Koirala B, Giri BR, Lamichhane A. Impact of revised code NBC105 on assessment and design of low rise reinforced concrete buildings in Nepal. *J Inst Eng.* 2021;16(1):1–5.
- [16] Rana, Sumit. 2021. “Advancement in Construction Techniques of Earthquake Resistant Buildings.” *8(7):936–41*.
- [17] Liu, W., Jiang, H., & Xiao, Y. (2020). Comparative study on seismic design codes of India, China, and Turkey. *Journal of Seismology and Earthquake Engineering*, 22(2), 123–136. <https://doi.org/10.1007/s10950-020-0982-5>
- [18] Uikey, Sangeeta, Rajiv Gandhi, Proudhyogiki Vishwavidyalaya, and Er Rahul Satbhaiya. 2020. “Seismic Response Analysis of Tall Building Using STAAD Pro Software.” (February). doi: 10.5281/zenodo.3685732.
- [19] Izhar, Tabish, Samreen Bano, and Neha Mumtaz. 2019. “Comparative Study on Analysis and Design of Reinforced Concrete Building under Seismic Forces for Different Codal Guidelines.” *International Journal of Trend in Scientific Research and Development Volume-3(Issue-4):536–51*. doi: 10.31142/ijtsrd23819.
- [20] RODRIGUES, R. A., C. E. N. MAZZILLI, and T. N. BITTENCOURT. 2019. “Comparative Analysis of Normative Provisions for Seismic Design and Detailing of Reinforced Concrete Structures.” *Revista IBRACON de Estruturas e Materiais* 12(5):1220–47. doi: 10.1590/s1983-41952019000500013.
- [21] Yashwant, Chaudhari Suraj, Patil Atit Ganesh, Patil Lawesh Shivnath, and Shelar Sachin Gurunath. 2018. “Review of Various Aspects of Seismically Safe Tall Buildings.” *4(1):1026–30*.
- [22] Ministry of Public Works and Settlement. (2018). *Specification for structures to be built in disaster areas (Turkish Earthquake Code 2018)*. Government of Turkey.
- [23] Fajfar, Peter. 2018. *Analysis in Seismic Provisions for Buildings: Past, Present and Future: The Fifth Prof. Nicholas Ambraseys Lecture*. Vol. 16. Springer Netherlands.

- [24] Nemitlu ÖF, Sari A. Comparison of Turkish Earthquake Code in 2007 With Turkish Earthquake Code in 2018, In: International Engineering and Natural Sciences Conference (IENSC 2018), Diyarbakir, Turkey, 2018.
- [25] Roy A, Purohit R. The Himalayas: evolution through collision. In: Indian shield: precambrian evolution and phanerozoic reconstitution, Amsterdam, Elsevier, 2018, pp. 311-327.
- [26] Nemitlu ÖF, Sari A. Comparison of Turkish Earthquake Code in 2007 With Turkish Earthquake Code in 2018, In: International Engineering and Natural Sciences Conference (IENSC 2018), Diyarbakir, Turkey, 2018.
- [27] Aksoylu C, Mobark A, Arslan MH, Erkan IH. A comparative study on ASCE 7-16, TBEC-2018 and TEC-2007 for reinforced concrete buildings. *Revista de la Construcción*. 2020;19(2):282–305.
- [28] Chopra, A. K. (2017). *Dynamics of structures: Theory and applications to earthquake engineering* (5th ed.). Pearson.
- [29] Santos, S. H. C., C. Giarelis, M. Traykova, C. Bucur, L. Zanaica, S. S. Lima, and S. S. Lima. 2017. "Comparative Study of a Set of Codes for the Seismic Design of Buildings." IABSE Conference, Vancouver 2017: Engineering the Future - Report 109(January):136–43. doi: 10.2749/vancouver.2017.0136.
- [30] Karaşin İB, Bakir D, Ülker M, Ulu AE. The structural damages after nepal earthquakes. *IOSRJEN*. 2017;07(06):45–54.
- [31] Yu G, Chock GYK, Comparison of the USA, China and Japan Seismic Design Procedures. In: Civil Engineering Conference in the Asia Region CECAR 7, Honolulu, Hawaii, 2016.
- [32] Yang, J., & Shi, B. (2016). Advances in seismic analysis of tall buildings: A review. *Earthquake Engineering and Structural Dynamics*, 45(8), 1231–1250. <https://doi.org/10.1002/eqe.2704>
- [33] Jones, Samantha, Katie J. Oven, and Ben Wisner. 2016. "A Comparison of the Governance Landscape of Earthquake Risk Reduction in Nepal and the Indian State of Bihar." *International Journal of Disaster Risk Reduction* 15:29–42. doi: 10.1016/j.ijdr.2015.10.011.
- [34] GB 50011-2010. (2016). Code for seismic design of buildings. Ministry of Housing and Urban-Rural Development of the People's Republic of China.
- [35] American Society of Civil Engineers. (2016). Minimum design loads for buildings and other structures (ASCE/SEI 7-16). ASCE.
- [36] Bureau of Indian Standards. (2016). Criteria for earthquake resistant design of structures - Part 1: General provisions and buildings (IS 1893:2016). BIS.
- [37] Taranath, B. S. (2016). *Structural analysis and design of tall buildings: Steel and composite construction* (2nd ed.). CRC Press.
- [38] Ohsumi T, Mukai Y, Fujitani H. Investigation of damage in and around kathmandu valley related to the 2015 Gorkha, Nepal Earthquake and Beyond. *Geotech Geol Eng*. 2016;34:1223–45.
- [39] Dizhur D, Dhakal RP, Bothara J, Ingham JM. Building typologies and failure modes observed in the 2015 Gorkha (Nepal) earthquake. *Bull N Z Soc Earthq Eng*. 2016;49(2):211–32.
- [40] Ohsumi T, Mukai Y, Fujitani H. Investigation of damage in and around kathmandu valley related to the 2015 Gorkha, Nepal Earthquake and Beyond. *Geotech Geol Eng*. 2016;34:1223–45.
- [41] Gautam D, Bhetwal KK, Rodrigues H, Neupane P, Sanada Y, Observed damage patterns on buildings during 2015 Gorkha (Nepal) Earthquake. *New Technologies for Urban Safety of Mega Cities in Asia*.
- [42] Dizhur D, Dhakal RP, Bothara J, Ingham JM. Building typologies and failure modes observed in the 2015 Gorkha (Nepal) earthquake. *Bull N Z Soc Earthq Eng*. 2016;49(2):211–32.
- [43] Adhikari RK, Bhagat S, Wijeyewickrema A, Damage scenario of reinforced concrete buildings in the 2015 Nepal Earthquakes. In: 14th International Symposium on New Technology for Urban Safety of Mega Cities in Asia, USMCA 2015, Kathmandu, Nepal, 2015
- [44] Liu C, Fang D, Zhao L. Reflection on earthquake damage of buildings in 2015 Nepal earthquake and seismic measures for post-earthquake reconstruction. *Structures*. 2021;30:647–58.
- [45] Sucuoglu, H., & Akkar, S. (2014). *Basic earthquake engineering: From seismology to analysis and design*. Springer. <https://doi.org/10.1007/978-3-319-03182-8>
- [46] Santos, S. H. C., C. Giarelis, M. Traykova, S. S. Lima, C. Bucur, and W. H. Orrala. 2014. "Comparative Study of Some Seismic Codes for Building Design Regarding Criteria for Non-Linear Methods of Analysis." (June):1–12.
- [47] Audru, J. C., J. L. Vernier, B. Capdeville, and J. J. Salindre. 2013. "Preparedness Actions towards Seismic Risk Mitigation for the General Public in Martinique, French Lesser Antilles: A Mid-Term Appraisal." *Natural Hazards and Earth System Sciences* 13(8):2031–39. doi: 10.5194/nhess-13-2031-2013.
- [48] Khose VN, Singh Y, Lang DH. A comparative study of design base shear for RC buildings in selected seismic design codes. *Earthq Spectra*. 2012;28(3):1047–70.
- [49] Wang, C., & Wu, G. (2010). Seismic response of high-rise buildings under varying earthquake intensities.

- Earthquake Spectra, 26(3), 849–865. <https://doi.org/10.1193/1.3459156>
- [50] Dowrick, D. J. (2009). *Earthquake resistant design and risk reduction* (2nd ed.). Wiley.
- [51] Lu, X., Guan, H., & Yang, Z. (2009). Simplified seismic design method for high-rise buildings. *Journal of Building Structures*, 30(3), 56–63. <https://doi.org/10.3969/j.issn.1000-6869.2009.03.008>
- [52] Halis Gunel, M., and H. Emre Ilgin. 2007. “A Proposal for the Classification of Structural Systems of Tall Buildings.” *Building and Environment* 42(7):2667–75. doi: 10.1016/j.buildenv.2006.07.007.
- [53] Aksoylu C, Mobark A, Arslan MH, Erkan IH. A comparative sstudy on ASCE 7–16, TBEC-2018 and TEC-2007 for reinforced concrete buildings. *Revista de la Construcción*. 2020;19(2):282–305.
- [54] Nemitlu ÖF, Sari A. Comparison of Turkish Earthquake Code in 2007 With Turkish Earthquake Code in 2018, In: *International Engineering and Natural Sciences Conference (IENSC 2018)*, Diyarbakir, Turkey, 2018.
- [55] Ali, M. M., & Moon, K. S. (2007). Structural developments in tall buildings: Current trends and future prospects. *Architectural Science Review*, 50(3), 205–223. <https://doi.org/10.3763/asre.2007.5027>
- [56] Priestley, M. J. N., Calvi, G. M., & Kowalsky, M. J. (2007). *Displacement-based seismic design of structures*. IUSS Press.
- [57] Lee, H. S., & Ko, D. E. (2007). Evaluation of seismic design methods for reinforced concrete structures. *Structural Engineering and Mechanics*, 26(1), 25–38. <https://doi.org/10.12989/sem.2007.26.1.025>
- [58] Agarwal, P., & Shrikhande, M. (2006). *Earthquake-resistant design of structures*. PHI Learning.
- [59] Doğangün A, Livaoğlu R. A comparative study of the design spectra defined by Eurocode 8, UBC, IBC and Turkish Earthquake Code on R/C sample buildings. *J Seismolog*. 2006;10:335–51.
- [60] Kaushik HB, Rai DC, Jain SK. Code approaches to seismic design of masonry-infilled reinforced concrete frames: a state-of-the-art review. *Earthq Spectra*. 2006;22(4):961–83.
- [61] Federal Emergency Management Agency. (2005). *NEHRP recommended provisions for seismic regulations for new buildings and other structures (FEMA 450)*. FEMA.
- [62] Eurocode 8. (2004). *Design of structures for earthquake resistance - Part 1: General rules, seismic actions, and rules for buildings*. European Committee for Standardization.
- [63] Bozorgnia, Y., & Bertero, V. V. (2004). *Earthquake engineering: From engineering seismology to performance-based engineering*. CRC Press.
- [64] Kim, S. J., & Kim, J. (2001). Seismic performance evaluation of multi-story buildings. *Engineering Structures*, 23(3), 333–344. [https://doi.org/10.1016/S0141-0296\(00\)00055-1](https://doi.org/10.1016/S0141-0296(00)00055-1)
- [65] Pinho, R., & Elnashai, A. S. (2000). Dynamic analysis of inelastic structures using the finite element method. *Computers & Structures*, 77(4), 551–562. [https://doi.org/10.1016/S0045-7949\(00\)00129-9](https://doi.org/10.1016/S0045-7949(00)00129-9)
- [66] Naeim, F., & Kelly, J. M. (1999). *Design of seismic isolated structures: From theory to practice*. Wiley.
- [67] Krawinkler, H., & Seneviratna, G. D. P. K. (1998). Pros and cons of a pushover analysis of seismic performance evaluation. *Engineering Structures*, 20(4–6), 452–464. [https://doi.org/10.1016/S0141-0296\(97\)00092-8](https://doi.org/10.1016/S0141-0296(97)00092-8)
- [68] Raleigh, Zia. 1996. “High Performance Concretes an Annotated Bibliography (1989-1994).” (June).
- [69] Paulay, T., & Priestley, M. J. N. (1992). *Seismic design of reinforced concrete and masonry buildings*. Wiley.
- [70] Wolff, R., & Burkhart, F. (1992). Comparison of seismic codes in the U.S., Japan, and Europe. *Journal of Structural Engineering*, 118(4), 1121–1135. [https://doi.org/10.1061/\(ASCE\)0733-9445\(1992\)118:4\(1121\)](https://doi.org/10.1061/(ASCE)0733-9445(1992)118:4(1121))
- [71] Smith, B. S., & Coull, A. (1991). *Tall building structures: Analysis and design*. Wiley.
- [72] Luft RW. Comparisons among earthquake codes. *Earthq Spectra*. 1989;5(4):767–89